



Wind Load Report - 1320

1. Site & Building Data

Roof Type: Gable
 Wind Speed (ult): 150 mph
 Exposure Category: C
 Enclosure Class: Enclosed
 Building Width (W): 60 ft.
 Building Length (L): 180 ft.
 Eave Height (h_e): 16 ft.
 Foundation Height (h_f): 1 ft.
 Roof Pitch: 6 /12
 Eave Overhang (OH_e): 2.5 ft.
 Gable Overhang (OH_g): 2.5 ft.

2. Parameters & Coefficients

Topographic Factor (K_{zt}): 1.0
 Directionality Factor (K_d): .85
 Roof Angle (θ): 26.57 deg.
 Mean Roof Height (h): 23.50 ft.
 Ridge Height (h_r): 31.00 ft.
 Pos. Internal Pressure (+GC_{pi}): +0.18
 Neg. Internal Pressure (-GC_{pi}): -0.18
 Velocity Pressure Exp. Coeff. (K_h): 0.93 @ z=h
 Velocity Pressure (q_h): 45.68 psf
 End Zone Width (a): 3.00 ft.
 Zone 2/2E Dist.: 30.00 ft.

3. Design Assumptions and Notes

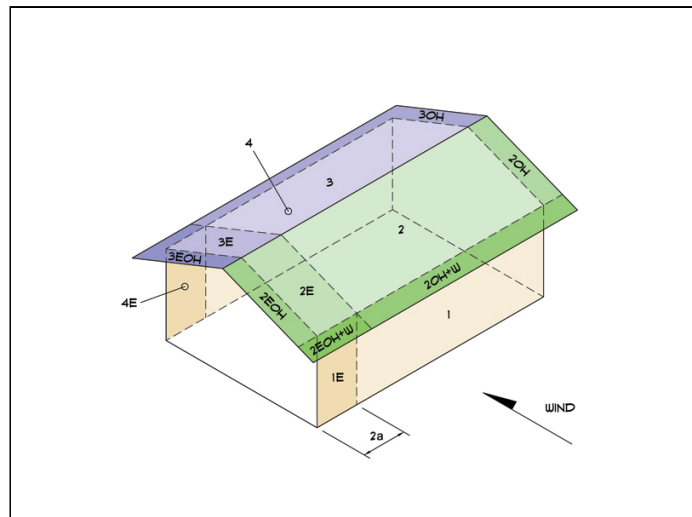
Code Standard: ASCE 7-10
 Geometry: Regular-Shaped Bldg.
 Height Class: Low-Rise Building
 Notes: big barn

4. Design Loads

Top Chord Dead Load: 7 psf
 Bottom Chord Dead Load: 10 psf
 Truss/Rafter Spacing: 48 in. o/c

4. Design Wind Pressures: MWFRS Envelope Procedure

Load Case A: Transverse Direction			
Surface	GC _{pf}	Design Pressure (psf)	
		(w/ +GC _{pi})	(w/ -GC _{pi})
1	0.55	16.89	33.33
2	-0.10	-12.75	3.69
3	-0.45	-28.65	-12.20
4	-0.39	-26.07	-9.62
1E	0.73	25.02	41.47
2E	-0.19	-16.91	-0.47
3E	-0.58	-34.94	-18.50
4E	-0.53	-32.66	-16.21
2OH	-0.10	-4.53	
2EOH	-0.19	-8.69	
3OH	-0.45	-20.43	
3EOH	-0.58	-26.72	
2OH+W	-0.10/-0.7	-34.02	
2EOH+W	-0.19/-0.7	-38.18	



- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
- b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
- c) Design building for all wind directions, 4 load patterns per load case.
- d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
- e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
- f) Design pressures are for strength design, multiply by 0.6 for ASD.

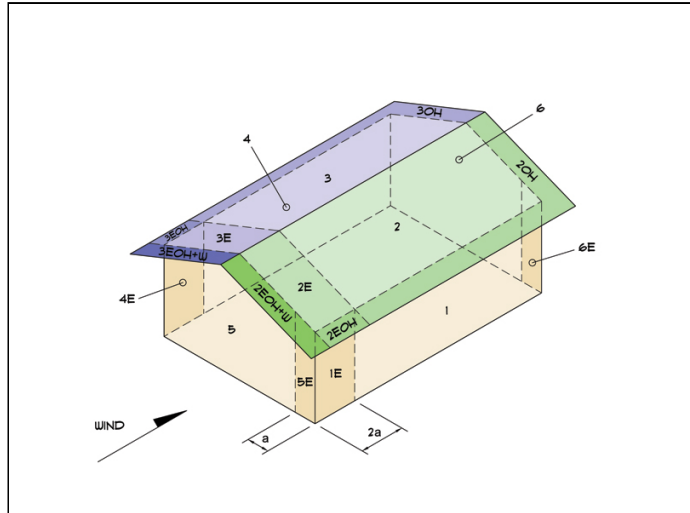
Subject Wind Loads	Customer Terrence Banning	Location 1320 Huntsville Rd	Job No. 1
Engr. Engineer Name	ENGINEERING COMPANY INC.		Rev. -
Date 4/13/2026	Street Address City, CA 99999 ph. (800) 000-0000 www.website.com		Page 1



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Load Case B: Longitudinal Direction

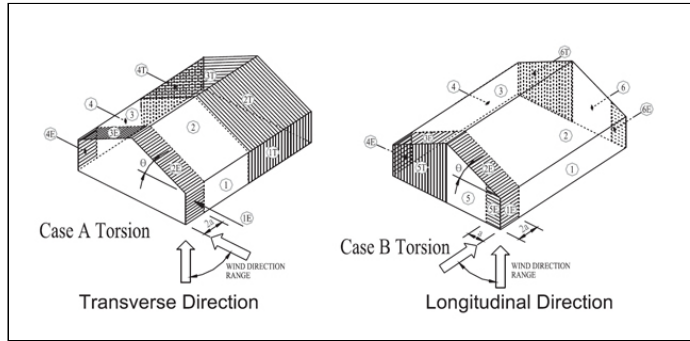
Surface	GCpf	Design Pressure (psf)	
		(w/ +GCpi)	(w/ -GCpi)
1	-0.45	-28.78	-12.33
2	-0.69	-39.74	-23.30
3	-0.37	-25.12	-8.68
4	-0.45	-28.78	-12.33
5	0.40	10.05	26.50
6	-0.29	-21.47	-5.02
1E	-0.48	-30.15	-13.70
2E	-1.07	-57.10	-40.66
3E	-0.53	-32.43	-15.99
4E	-0.48	-30.15	-13.70
5E	0.61	19.64	36.09
6E	-0.43	-27.87	-11.42
2OH	-0.69		-31.52
2EOH	-1.07		-48.88
3OH	-0.37		-16.90
3EOH	-0.53		-24.21
2EOH+W	-1.07/-0.7		-80.86
3EOH+W	-0.53/-0.7		-56.19



- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
- b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
- c) Design building for all wind directions, 4 load patterns per load case.
- d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
- e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
- f) Design pressures are for strength design, multiply by 0.6 for ASD.

Torsional Load Cases

Surface	Load Case	GCpf	Design Pressure (psf)	
			(w/ +GCpi)	(w/ -GCpi)
1T	A	-	4.22	8.33
2T	A	-	-3.19	0.92
3T	A	-	-7.16	-3.05
4T	A	-	-6.52	-2.41
5T	B	-	2.51	6.62
6T	B	-	-5.37	-1.26



- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
- b) Pressures designated with a "T" are 25% of full design wind pressures.
- c) Torsional loading shall apply to all 8 load patterns using the figures shown.
- d) Design pressures are for strength design, multiply by 0.6 for ASD.
- e) Torsional Design Exceptions: One story bldg. with $h \leq 30$ ft.
Two stories or less framed with light frame construction,
Two stories or less with flexible diaphragms.

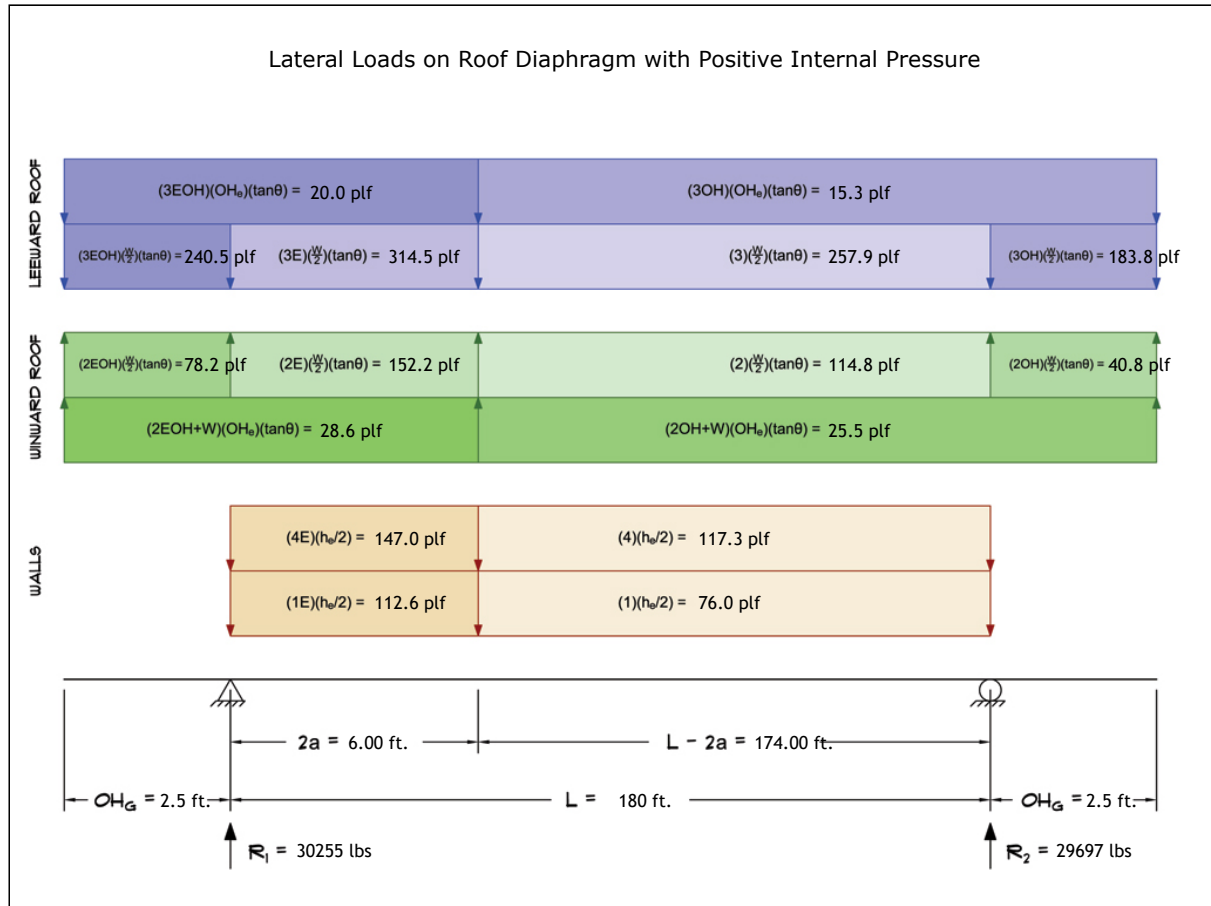
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5. Wind Load Calculations

1.) Lateral Loads - Transverse Direction:



- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

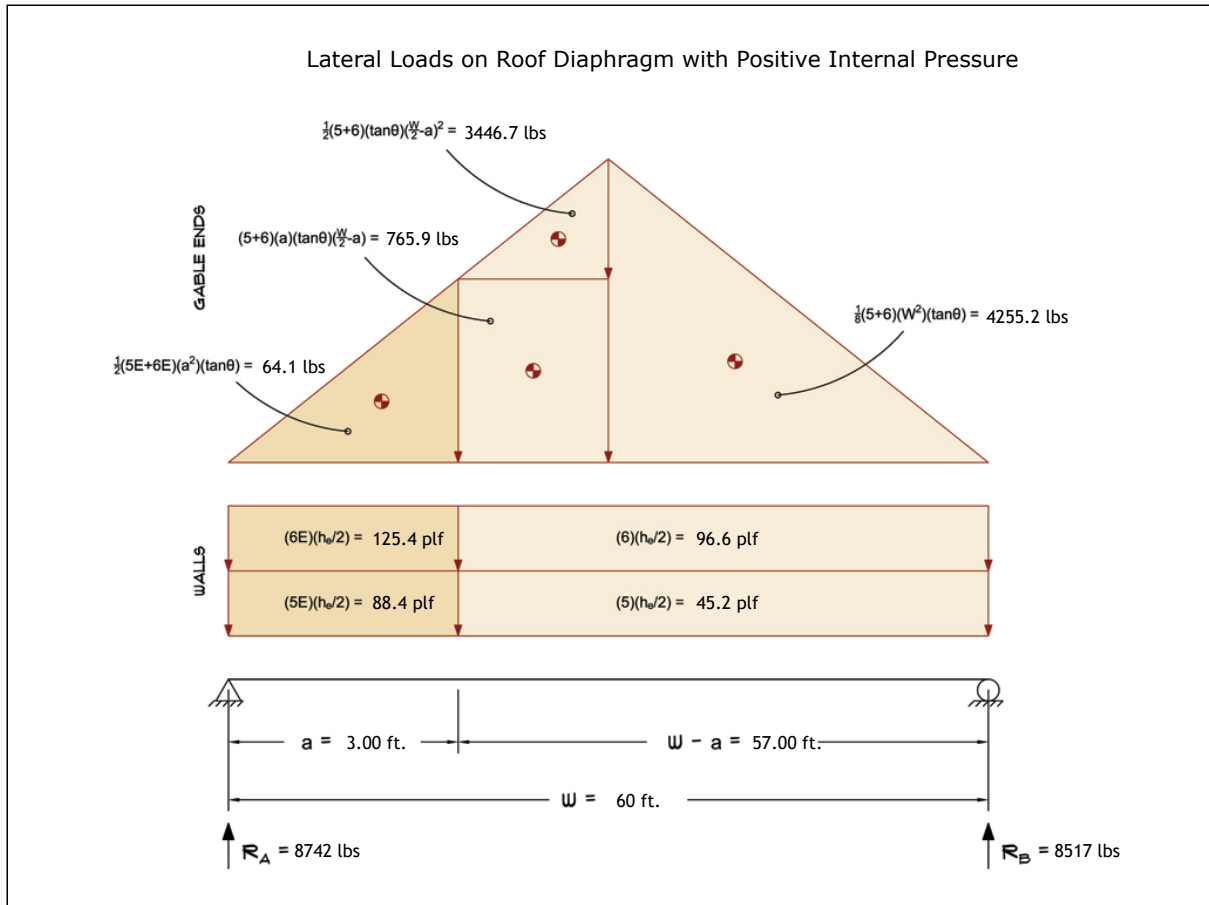
Wind Base Shear (ASD)						
Load Case A: Transverse Direction						
Load Case	Walls (lbs)	Roof (lbs)	Roof Overhangs (lbs)	Total Lateral Load (lbs)	R ₁ (lbs)	R ₂ (lbs)
Positive Internal Pressure	35191	25870	-1109	59952	30255	29697
Negative Internal Pressure	35191	25870	-1109	59952	30255	29697
Roof Pressure = 0	35191	0	0	35191	17788	17403
Min. Pressures (8 psf, 16 psf)	12960	12960	1470	27390	13695	13695

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

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2.) Lateral Loads - Longitudinal Direction:



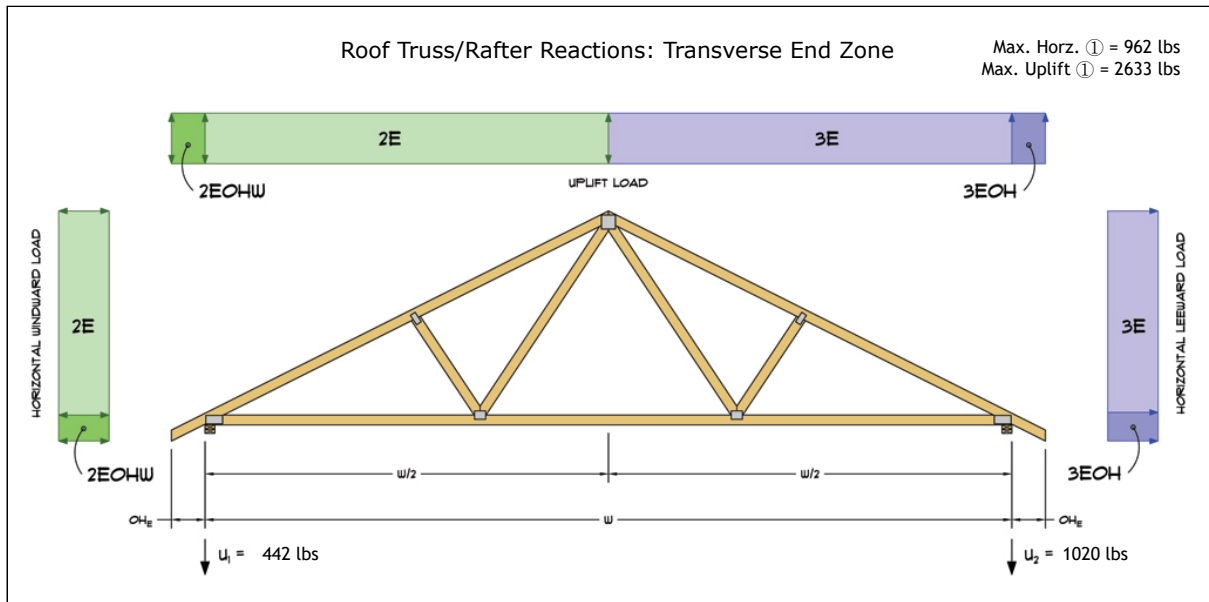
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.
- c) Where the length of building (L) exceeds 4X the mean roof height (h), wind drag forces should additionally be considered.

Wind Base Shear (ASD)						
Load Case B: Longitudinal Direction						
Load Case	Walls (lbs)	Gable Ends (lbs)	Roof (lbs)	Total Lateral Load (lbs)	R _A (lbs)	R _B (lbs)
Positive Internal Pressure	8726	8532	0	17258	8742	8517
Negative Internal Pressure	8726	8532	0	17258	8742	8517
Roof Pressure = 0	8726	8532	0	17258	8742	8517
Min. Pressures (8 psf, 16 psf)	4320	4320	0	8640	4320	4320

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

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3.) Roof Truss Reactions:



- a) Strength design values multiplied by 0.6 to obtain ASD values.
- b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)					
w/ Positive Internal Pressure					
Load Case	Horizontal Load (lbs)	Gross Uplift (lbs)	Net Uplift (lbs)	U ₁ (lbs)	U ₂ (lbs)
Transverse Int. Zone	-532	3308	647	80	567
Transverse End Zone	-615	4123	1462	442	1020
Longitudinal Int. Zone	570	4961	2300	1459	841
Longitudinal End Zone	962	6885	4224	2633	1591

- a) Gross Uplift calculations do not include any counteracting roof dead loads.
- b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10.
- c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads.
- d) Loads based on truss spacing calculated at 48" o/c.
- e) Negative values for horizontal load indicate load acting in windward direction (transverse load cases).
- f) Negative values for uplift indicate net downward force (zero uplift).

*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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