

# Wind Load Report

### 1. Site & Building Data

#### Roof Type: Gable Wind Speed (ult): 115 mph C **Exposure Category: Enclosure Class:** Enclosed 28 ft. Building Width (W): 48 ft. Building Length (L): Eave Height (he): 9.1 ft. .5 ft. Foundation Height (h<sub>f</sub>): Roof Pitch: 4 /12 Eave Overhang (OHe): 2 ft. Gable Overhang (OH<sub>g</sub>): 2.5 ft.

### 2. Parameters & Coefficients

Topographic Factor (Kzt):	1.0	
Directionality Factor (Kd):	.85	
Roof Angle ( $\theta$ ):	18.43	deg.
Mean Roof Height (h):	11.43	ft.
Ridge Height (h <sub>r</sub> ):	13.77	ft.
Pos. Internal Pressure (+GCpi):	+0.18	
Neg. Internal Pressure (-GCpi):	-0.18	
Velocity Pressure Exp. Coeff. (K <sub>h</sub> ):	0.85	@ z=h
Velocity Pressure (qh):	24.43	psf
End Zone Width (a):	3.00	ft.

14.00 ft.

### 3. Design Assumptions and Notes

Code Standard: **ASCE 7-10** Geometry: Regular-Shaped Bldg. Height Class: Low-Rise Building

Notes:

### 4. Design Loads

Zone 2/2E Dist.:

Top Chord Dead Load: 14 psf Bottom Chord Dead Load: 10 psf Truss/Rafter Spacing: 24 in. o/c

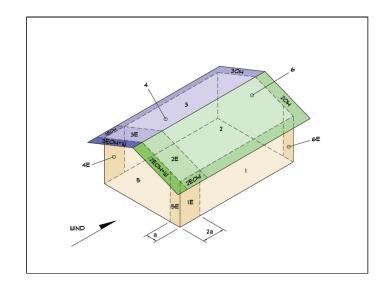
# 4. Design Wind Pressures: MWFRS Envelope Procedure

Load Case A: Transverse Direction					
Surface	CCf	Design Pressure (psf)			
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)		
1	0.52	8.22	17.01		
2	-0.69	-21.25	-12.46		
3	-0.47	-15.84	-7.05		
4	-0.42	-14.54	-5.75		
1E	0.78	14.66	23.46		
2E	-1.07	-30.54	-21.74		
3E	-0.67	-20.85	-12.05		
4E	-0.62	-19.50	-10.70		
2OH	-0.69	-16	.86		
2EOH	-1.07	-26	.14		
3ОН	-0.47	-11.45			
3ЕОН	-0.67	-16.45			
2OH+W	-0.69/-0.7	-33.96			
2EOH+W	-1.07/-0.7	-43.24			

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- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10. c) Design building for all wind directions, 4 load patterns per load case.
- d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
- e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof. f) Design pressures are for strength design, multiply by 0.6 for ASD.
- Subject Customer Location Wind Loads 734 This report may not be copied, reproduced or distributed without the written consent of **Engineer Name** ENGINEERING COMPANY INC. Date Street Address City, CA 99999 Page 4/3/2025 ph. (800) 000-0000 www.website.com

Load Case B: Longitudinal Direction						
Surface	CC-f	Design Pressure (psf)				
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)			
1	-0.45	-15.39	-6.60			
2	-0.69	-21.25	-12.46			
3	-0.37	-13.44	-4.64			
4	-0.45	-15.39	-6.60			
5	0.40	5.37	14.17			
6	-0.29	-11.48	-2.69			
1E	-0.48	-16.12	-7.33			
2E	-1.07	-30.54	-21.74			
3E	-0.53	-17.34	-8.55			
4E	-0.48	-16.12	-7.33			
5E	0.61	10.50	19.30			
6E	-0.43	-14.90	-6.11			
2OH	-0.69	-16	.86			
2EOH	-1.07	-26	.14			
3ОН	-0.37	-9.	.04			
3ЕОН	-0.53	-12	95			
2EOH+W	-1.07/-0.7	-43.24				
3EOH+W	-0.53/-0.7	-30.05				

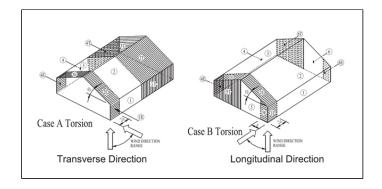


- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
  b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
  c) Design building for all wind directions, 4 load patterns per load case.
  d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
  e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
  f) Design pressures are for strength design, multiply by 0.6 for ASD.

Torsional Load Cases								
Surface	T 10	GCpf	Design Pressure (psf)					
Surface	Load Case	(w	(w/+GCpi)	(w/ -GCpi)				
1T	A	-	2.05	4.25				
2T	A	-	-5.31	-3.11				
3T	A	-	-3.96	-1.76				
4T	A	-	-3.64	-1.44				
5T	В	-	1.34	3.54				
6T	В	-	-2.87	-0.67				

- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) Pressures designated with a "T" are 25% of full design wind pressures. c) Torsional loading shall apply to all 8 load patterns using the figures shown. d) Design pressures are for strength design, multiply by 0.6 for ASD. e) Torsional Design Exceptions. One story bldg. with  $h \leq 30 \, \text{ft},$

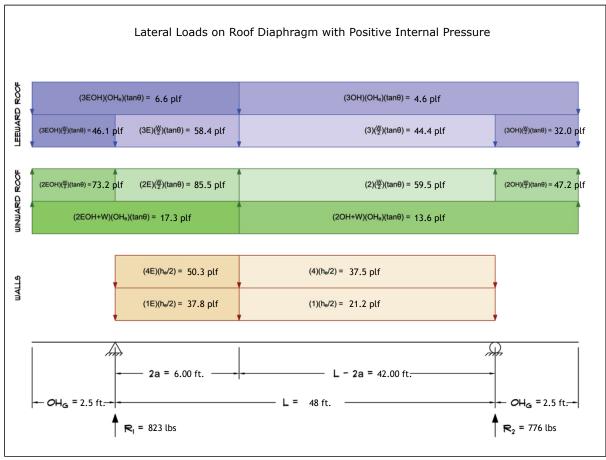
- Two stories or less framed with light frame construction, Two stories or less with flexible diaphragms.



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## 5. Wind Load Calculations

#### 1.) <u>Lateral Loads - Transverse Direction</u>:



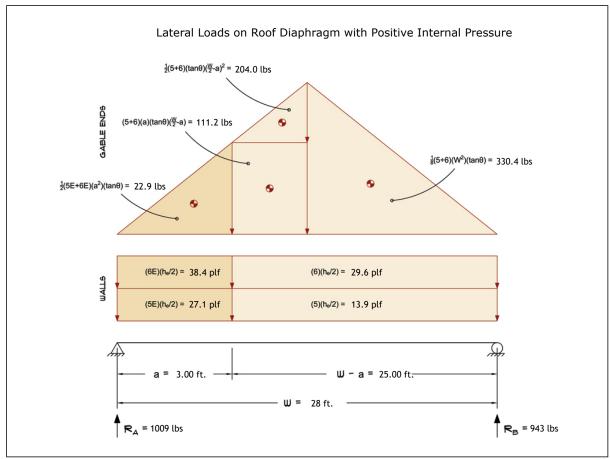
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

Wind Base Shear (ASD)							
Load Case A: Transverse Direction							
	Load	Case A: 1	ransverse Directi	on			
Load Case	Walls (lbs)	Roof (lbs)	Roof Overhangs (lbs)	Total Lateral Load (lbs)	R <sub>1</sub> (lbs)	R <sub>2</sub> (lbs)	
Positive Internal Pressure	2995	-799	-597	1599	823	776	
Negative Internal Pressure	2995	-799	-597	1599	823	776	
Roof Pressure = 0	2995	0	0	2995	1575	1421	
Min. Pressures (8 psf, 16 psf)	1981	1075	282	3338	1669	1669	

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

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#### 2.) <u>Lateral Loads - Longitudinal Direction</u>:



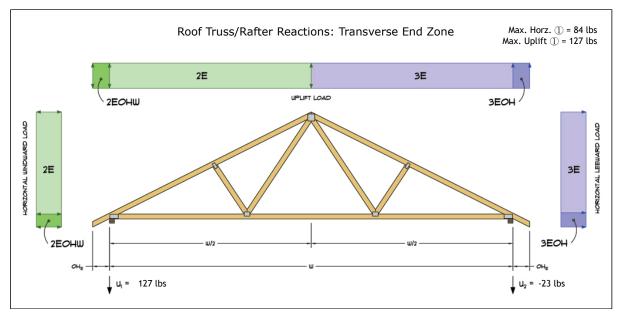
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
  b) Strength design values multiplied by 0.6 to obtain ASD values.
  c) Where the length of building (L) exceeds 4X the mean roof height (h), wind drag forces should additionally be considered.

Wind Base Shear (ASD)						
	Load Case B: Longitudinal Direction					
Load Case	Walls (lbs)	Gable Ends (lbs)	Roof (lbs)	Total Lateral Load (lbs)	R <sub>A</sub> (lbs)	R <sub>B</sub> (lbs)
Positive Internal Pressure	1284	668	0	1952	1009	943
Negative Internal Pressure	1284	668	0	1952	1009	943
Roof Pressure = 0	1284	668	0	1952	1009	943
Min. Pressures (8 psf, 16 psf)	1156	627	0	1783	892	892

- a) Bottom half of wall neglected in tributary area calculations.
   b) Strength design values multiplied by 0.6 to obtain ASD values.

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#### 3.) Roof Truss Reactions:



- a) Strength design values multiplied by 0.6 to obtain ASD values.
- b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)							
w/ Positive Internal Pressure							
Load Case	Horizontal Load (lbs)	Gross Uplift (lbs)	Net Uplift (lbs)	U <sub>1</sub> (lbs)	U <sub>2</sub> (lbs)		
Transverse Int. Zone	48	732	-171	-34	-137		
Transverse End Zone	76	1006	104	127	-23		
Longitudinal Int. Zone	50	645	-258	-86	-172		
Longitudinal End Zone	84	898	-4	70	-75		

- a) Gross Uplift calculations do not include any counteracting roof dead loads.
- b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10. c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads. d) Loads based on truss spacing calculated at 24" o/c.

- e) Negative values for horizontal load indicate load acting in windward direction (tranverse load cases).
- f) Negative values for uplift indicate net downward force (zero uplift).

\*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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