

# Wind Load Report - New ADU

## 1. Site & Building Data

#### Roof Type: Wind Speed (ult): 110 mph $\mathbf{C}$ Exposure Category: **Enclosure Class:** Enclosed 25 ft. Building Width (W): 46 ft. Building Length (L): Eave Height (he): 10 ft. Foundation Height (hf): 0 ft. Roof Pitch: 12 /12 Eave Overhang (OH<sub>e</sub>): 2 ft. Gable Overhang (OHg): 2 ft.

### 2. Parameters & Coefficients

Topographic Factor (Kzt):	1.0	
Directionality Factor (Kd):	.85	
Roof Angle ( $\theta$ ):	45.00	deg.
Mean Roof Height (h):	16.25	ft.
Ridge Height (h <sub>r</sub> ):	22.50	ft.
Pos. Internal Pressure (+GCpi):	+0.18	
Neg. Internal Pressure (-GCpi):	-0.18	
Velocity Pressure Exp. Coeff. $(K_h)$ :	0.86	@ z=h
Velocity Pressure (qh):	22.73	psf
End Zone Width (a):	3.00	ft.

12.50 ft.

### 3. Design Assumptions and Notes

Code Standard: **ASCE 7-10** Geometry: Regular-Shaped Bldg. Height Class: Low-Rise Building

Notes:

#### 4. Design Loads

Zone 2/2E Dist.:

Top Chord Dead Load: 7 psf Bottom Chord Dead Load: 10 psf 24 in. o/c Truss/Rafter Spacing:

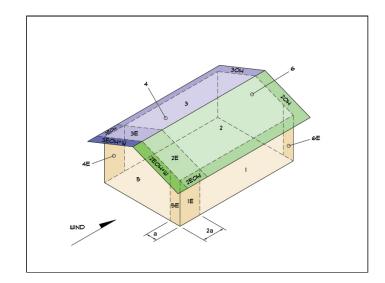
### 4. Design Wind Pressures: MWFRS Envelope Procedure

Load Case A: Transverse Direction						
Surface	CC-f	Design Pressure (psf)				
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)			
1	0.56	8.64	16.82			
2	0.21	0.68	8.86			
3	-0.43	-13.87	-5.68			
4	-0.37	-12.50	-4.32			
1E	0.69	11.59	19.78			
2E	0.27	2.05	10.23			
3E	-0.53	-16.14	-7.96			
4E	-0.48	-15.00	-6.82			
2OH	0.21	4.	77			
2EOH	0.27	6.	14			
3ОН	-0.43	-9.77				
3ЕОН	-0.53	-12.05				
2OH+W	0.21/-0.7	-10.87				
2EOH+W	0.27/-0.7	-9.	51			

- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10. c) Design building for all wind directions, 4 load patterns per load case.
- d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
- e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof. f) Design pressures are for strength design, multiply by 0.6 for ASD.

Subject	Customer	Location			Job No.
Wind Loads		985 Civ	vic Center Dr Santa Clara	a	50304
Engineer Name	ENGINEERING C		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
3/7/2025	Street Address City, CA 999 ph. (800) 000-0000 www.v		COMPANY LOGO	Copyright © 2025	Page 1

Load Case B: Longitudinal Direction						
C	CC-f	Design Pressure (psf)				
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)			
1	-0.45	-14.32	-6.14			
2	-0.69	-19.78	-11.59			
3	-0.37	-12.50	-4.32			
4	-0.45	-14.32	-6.14			
5	0.40	5.00	13.18			
6	-0.29	-10.68	-2.50			
1E	-0.48	-15.00	-6.82			
2E	-1.07	-28.41	-20.23			
3E	-0.53	-16.14	-7.96			
4E	-0.48	-15.00	-6.82			
5E	0.61	9.77	17.96			
6E	-0.43	-13.87	-5.68			
2OH	-0.69	-15	.68			
2EOH	-1.07	-24	.32			
3ОН	-0.37	-8.41				
3ЕОН	-0.53	-12.05				
2EOH+W	-1.07/-0.7	-40	0.23			
3EOH+W	-0.53/-0.7	-27.96				

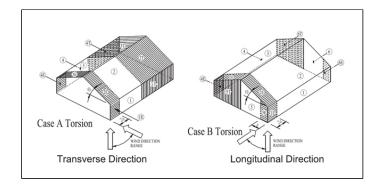


- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
  b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
  c) Design building for all wind directions, 4 load patterns per load case.
  d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
  e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
  f) Design pressures are for strength design, multiply by 0.6 for ASD.

Torsional Load Cases								
Surface	Load Case	GCpf	Design Pre					
Surface	Load Case	Сері (	(w/+GCpi)	(w/ -GCpi)				
1T	A	-	2.16	4.21				
2T	A	-	0.17	2.22				
3T	A	-	-3.47	-1.42				
4T	A	-	-3.13	-1.08				
5T	В	-	1.25	3.30				
6T	В	-	-2.67	-0.63				

- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) Pressures designated with a "T" are 25% of full design wind pressures. c) Torsional loading shall apply to all 8 load patterns using the figures shown. d) Design pressures are for strength design, multiply by 0.6 for ASD. e) Torsional Design Exceptions. One story bldg. with  $h \leq 30 \, \text{ft},$

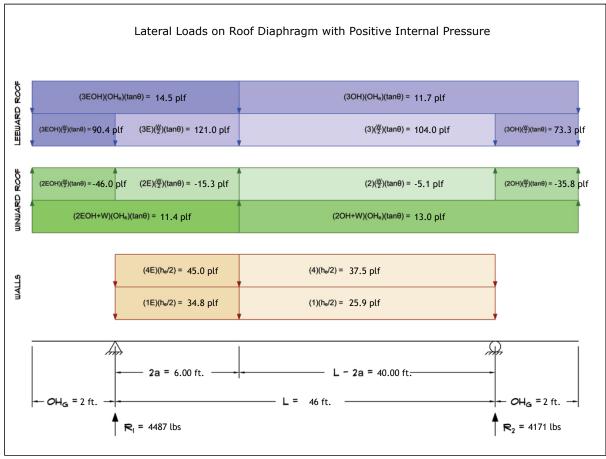
Two stories or less framed with light frame construction, Two stories or less with flexible diaphragms.



Subject	Customer	Location			Job No.
Wind Loads		985 C	Civic Center Dr Santa Clar	a	50304
Engr.					Rev.
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	-
3/7/2025	Street Address City, CA 999 ph. (800) 000-0000 www.v		COMPANY LOGO  Copyright ©		Page 2

### 5. Wind Load Calculations

#### 1.) <u>Lateral Loads - Transverse Direction</u>:



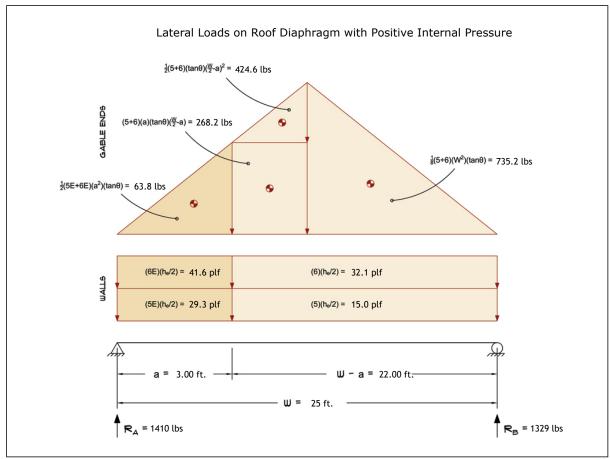
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

Wind Base Shear (ASD)							
Load Case A: Transverse Direction							
Load Case	Walls (lbs)	Roof (lbs)	Roof Overhangs (lbs)	Total Lateral Load (lbs)	R <sub>1</sub> (lbs)	R <sub>2</sub> (lbs)	
Positive Internal Pressure	3015	5183	460	8658	4487	4171	
Negative Internal Pressure	3015	5183	460	8658	4487	4171	
Roof Pressure = 0	3015	0	0	3015	1550	1465	
Min. Pressures (8 psf, 16 psf)	2208	2760	720	5688	2844	2844	

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

Subject Wind Loads	Customer	Location 985 Ci	Job No. 50304		
Engineer Name  Date  3/7/2025	ENGINEERING CO Street Address City, CA 999 ph. (800) 000-0000 www.v		STRUCTURAL ENGINEERS COMPANY LOGO	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.  Copyright © 2025	Page 3

#### 2.) <u>Lateral Loads - Longitudinal Direction</u>:



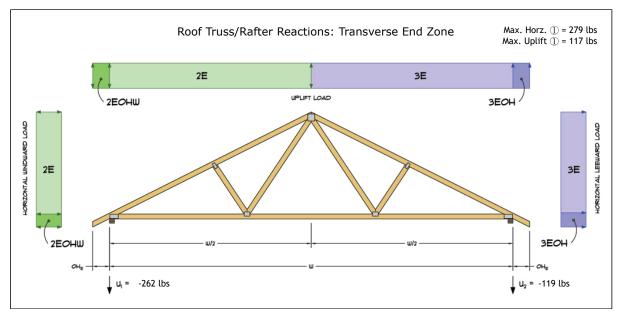
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
  b) Strength design values multiplied by 0.6 to obtain ASD values.
  c) Where the length of building (L) exceeds 4X the mean roof height (h), wind drag forces should additionally be considered.

Wind Base Shear (ASD)							
	Load Case B: Longitudinal Direction						
Load Case	Walls (lbs)	Gable Ends (lbs)	Roof (lbs)	Total Lateral Load (lbs)	R <sub>A</sub> (lbs)	R <sub>B</sub> (lbs)	
Positive Internal Pressure	1248	1492	0	2740	1410	1329	
Negative Internal Pressure	1248	1492	0	2740	1410	1329	
Roof Pressure = 0	1248	1492	0	2740	1410	1329	
Min. Pressures (8 psf, 16 psf)	1200	1500	0	2700	1350	1350	

- a) Bottom half of wall neglected in tributary area calculations.
   b) Strength design values multiplied by 0.6 to obtain ASD values.

Subject	Customer	Location			Job No.
Wind Loads		985 C	50304		
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
3/7/2025	Street Address City, CA 999 ph. (800) 000-0000 www.v	999 vebsite.com	COMPANY LOGO		Page 4

#### 3.) Roof Truss Reactions:



- a) Strength design values multiplied by 0.6 to obtain ASD values.
- b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)							
w/ Positive Internal Pressure							
Load Case	Horizontal Load (lbs)	Gross Uplift (lbs)	Net Uplift (lbs)	U <sub>1</sub> (lbs)	U <sub>2</sub> (lbs)		
Transverse Int. Zone	-216	247	-397	-252	-145		
Transverse End Zone	-279	263	-381	-262	-119		
Longitudinal Int. Zone	127	542	-103	-15	-88		
Longitudinal End Zone	214	756	111	117	-6		

- a) Gross Uplift calculations do not include any counteracting roof dead loads.
- b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10. c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads. d) Loads based on truss spacing calculated at 24" o/c.

- e) Negative values for horizontal load indicate load acting in windward direction (tranverse load cases).
- f) Negative values for uplift indicate net downward force (zero uplift).

\*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

Subject Wind Loads	Customer	Location 985 (	Civic Center Dr Santa Clara	ı	<sup>Јор No.</sup> 50304
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
3/7/2025	Street Address City, CA 9999 ph. (800) 000-0000 www.we	99 vebsite.com	COMPANY LOGO	Copyright © 2025	Page 5