Wind Load Report - Five units

1. Site & Building Data

2. Parameters & Coefficients

Gable Roof Type: Wind Speed (ult): 95 mph Exposure Category: \mathbf{C} **Enclosure Class:** Enclosed Building Width (W): 36 ft. 140 ft. Building Length (L): Eave Height (he): 9 ft. Foundation Height (h_f): 0 ft. Roof Pitch: 5 /12 Eave Overhang (OH_e): 1 ft. Gable Overhang (OHg): 1 ft.

Topographic Factor (K_{zt}): 1.0 Directionality Factor (K_d): .85 Roof Angle (θ): 22.62 deg. Mean Roof Height (h): 12.75 ft. Ridge Height (h_r): 16.50 ft. Pos. Internal Pressure (+GCpi): +0.18Neg. Internal Pressure (-GCpi): -0.18Velocity Pressure Exp. Coeff. (Kh): 0.85 @ z=h Velocity Pressure (qh): 16.67 psf

End Zone Width (a): 3.00 ft. Zone 2/2E Dist.: 18.00 ft.

3. Design Assumptions and Notes

4. Design Loads

Code ASCE 7-10 Standard:

Top Chord Dead Load: 7 psf Bottom Chord Dead Load: 10 psf

Regular-Shaped Bldg. Truss/Rafter Spacing: Geometry:

24 in. o/c

Height

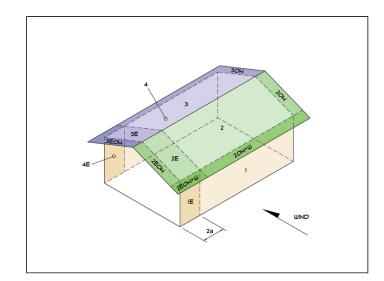
Low-Rise Building Class:

Notes: This is for the five unit structure. A

separate evaluation will be made for the single unit.

4. Design Wind Pressures: MWFRS Envelope Procedure

Load C	Load Case A: Transverse Direction						
Surface	GCpf	Design Pressure (psf)					
Surface	ССРІ	(w/ +GCpi)	(w/ -GCpi)				
1	0.54	5.97	11.97				
2	-0.45	-10.57	-4.57				
3	-0.47	-10.78	-4.78				
4	-0.41	-9.91	-3.91				
1E	0.77	9.86	15.86				
2E	-0.72	-14.99	-8.98				
3E	-0.65	-13.80	-7.80				
4E	-0.60	-12.97	-6.97				
2OH	-0.45	-7.	.57				
2EOH	-0.72	-11	.99				
3ОН	-0.47	-7.78					
3ЕОН	-0.65	-10.80					
2OH+W	-0.45/-0.7	-19.24					
2EOH+W	-0.72/-0.7	-23	.65				



- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
- b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.

c) Design building for all wind directions, 4 load patterns per load case.
d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
Subject
e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof. f) Design pressures are for strength design, multiply by 0.6 for ASD.

Job No

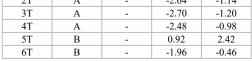
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Load Case B: Longitudinal Direction						
C	CC-f	Design Pressure (psf)				
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)			
1	-0.45	-10.50	-4.50			
2	-0.69	-14.50	-8.50			
3	-0.37	-9.17	-3.17			
4	-0.45	-10.50	-4.50			
5	0.40	3.67	9.67			
6	-0.29	-7.84	-1.83			
1E	-0.48	-11.00	-5.00			
2E	-1.07	-20.84	-14.84			
3E	-0.53	-11.84	-5.83			
4E	-0.48	-11.00	-5.00			
5E	0.61	7.17	13.17			
6E	-0.43	-10.17	-4.17			
2OH	-0.69	-11	.50			
2EOH	-1.07	-17	.84			
3ОН	-0.37	-6.17				
3ЕОН	-0.53	-8.84				
2EOH+W	-1.07/-0.7	-29.51				
3EOH+W	-0.53/-0.7	-20	.50			



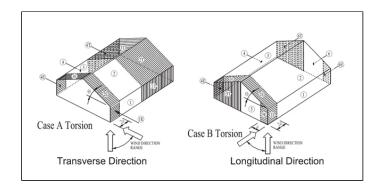
- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
 b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
 c) Design building for all wind directions, 4 load patterns per load case.
 d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
 e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
 f) Design pressures are for strength design, multiply by 0.6 for ASD.

Torsional Load Cases							
Surface	Load Case	GCpf	Design Pressure (psf)				
Surface	Load Case	ССРІ	(w/+GCpi)	(w/ -GCpi)			
1T	A	-	1.49	2.99			
2T	A	-	-2.64	-1.14			
3T	A	-	-2.70	-1.20			
4T	A	-	-2.48	-0.98			
5T	В	-	0.92	2.42			
6T	В	-	-1.96	-0.46			



- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) Pressures designated with a "T" are 25% of full design wind pressures. c) Torsional loading shall apply to all 8 load patterns using the figures shown. d) Design pressures are for strength design, multiply by 0.6 for ASD. e) Torsional Design Exceptions. One story bldg. with $h \leq 30 \, \text{ft},$

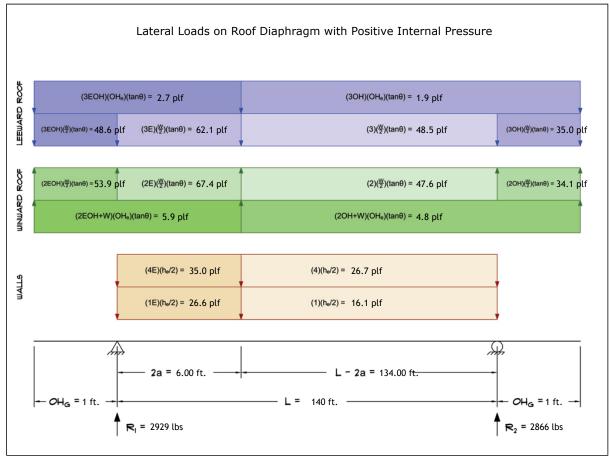
Two stories or less framed with light frame construction, Two stories or less with flexible diaphragms.



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5. Wind Load Calculations

1.) <u>Lateral Loads - Transverse Direction</u>:



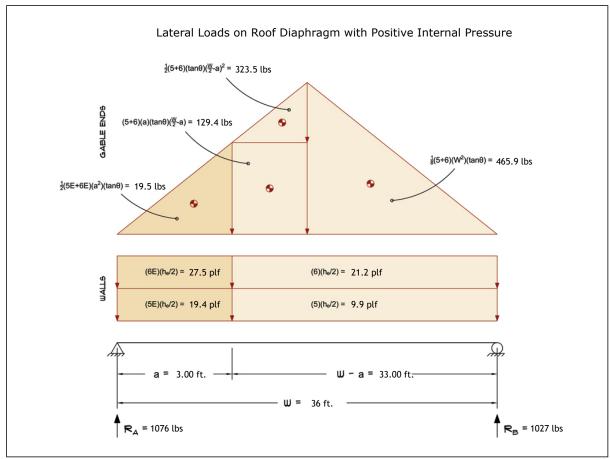
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

Wind Base Shear (ASD)								
Load Case A: Transverse Direction								
Load Case	Load Case Walls (lbs) Roof (lbs) Roof Overhangs (lbs) Total Lateral Load (lbs) R1 (lbs) R2 (lbs)							
Positive Internal Pressure 6113 96 -414 5795 2929						2866		
Negative Internal Pressure	Negative Internal Pressure 6113 96 -414 5795 2929 286							
Roof Pressure = 0 6113 0 0 6113 3110 3000								
Min. Pressures (8 psf, 16 psf)	6048	5040	356	11444	5722	5722		

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

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2.) <u>Lateral Loads - Longitudinal Direction</u>:



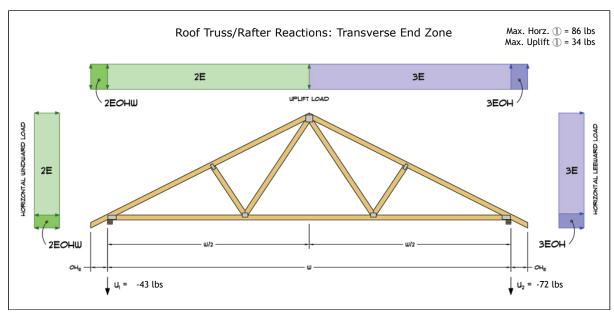
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
 b) Strength design values multiplied by 0.6 to obtain ASD values.
 c) Where the length of building (L) exceeds 4X the mean roof height (h), wind drag forces should additionally be considered.

Wind Base Shear (ASD)								
	Load Case B: Longitudinal Direction							
Load Case	Load Case Walls (lbs) Gable Ends (lbs) Roof (lbs) Total Lateral Load (lbs) RA (lbs) RB (
Positive Internal Pressure	Positive Internal Pressure 1165 938 0 2104 1076							
Negative Internal Pressure	Negative Internal Pressure 1165 938 0 2104 1076 10							
Roof Pressure = 0	Roof Pressure = 0 1165 938 0 2104 1076 102							
Min. Pressures (8 psf, 16 psf)	1555	1296	0	2851	1426	1426		

- a) Bottom half of wall neglected in tributary area calculations.
 b) Strength design values multiplied by 0.6 to obtain ASD values.

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3.) Roof Truss Reactions:



- a) Strength design values multiplied by 0.6 to obtain ASD values.
- b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)						
w/ Positive Internal Pressure						
Load Case Horizontal Load (lbs) Gross Uplift (lbs) Net Uplift (lbs) U1 (lbs) U2 (lb						
Transverse Int. Zone	4	494	-284	-136	-148	
Transverse End Zone 17 663 -115 -43 -72						
Longitudinal Int. Zone 51 533 -245 -91 -1						
Longitudinal End Zone	86	738	-40	34	-74	

- a) Gross Uplift calculations do not include any counteracting roof dead loads.
- b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10. c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads. d) Loads based on truss spacing calculated at 24" o/c.

- e) Negative values for horizontal load indicate load acting in windward direction (tranverse load cases).
- f) Negative values for uplift indicate net downward force (zero uplift).

*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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