

# Wind Load Report

### 1. Site & Building Data

#### Roof Type: Gable Wind Speed (ult): 180 mph В **Exposure Category: Enclosure Class:** Enclosed 38 ft. Building Width (W): Building Length (L): 45 ft. Eave Height (he): 8 ft. Foundation Height (h<sub>f</sub>): 0 ft. Roof Pitch: 8 /12 Eave Overhang (OHe): 1 ft. Gable Overhang (OH<sub>g</sub>): 1 ft.

#### 2. Parameters & Coefficients

Topographic Factor (K <sub>zt</sub> ):	1.0
Directionality Factor (K <sub>d</sub> ):	.85
Roof Angle ( $\theta$ ):	33.69 deg.
Mean Roof Height (h):	14.33 ft.
Ridge Height (h <sub>r</sub> ):	20.67 ft.
Pos. Internal Pressure (+GCpi):	+0.18
Neg. Internal Pressure (-GCpi):	-0.18
Velocity Pressure Exp. Coeff. (K <sub>h</sub> ):	0.70 @ z=h
Velocity Pressure (qh):	49.39 psf
End Zone Width (a):	3.00 ft.

19.00 ft.

# 3. Design Assumptions and Notes

Code Standard: **ASCE 7-10** Geometry: Regular-Shaped Bldg. Height Class: Low-Rise Building

Notes:

### 4. Design Loads

Zone 2/2E Dist.:

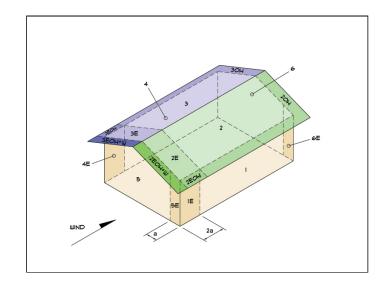
Top Chord Dead Load: 10 psf Bottom Chord Dead Load: 10 psf Truss/Rafter Spacing: 16 in. o/c

# 4. Design Wind Pressures: MWFRS Envelope Procedure

Load Case A: Transverse Direction					
Surface	CC-f	Design Pressure (psf)			
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)		
1	0.56	18.77	36.55		
2	0.21	1.48	19.26		
3	-0.43	-30.13	-12.35		
4	-0.37	-27.17	-9.38		
1E	0.69	25.19	42.97		
2E	0.27	4.45	22.23		
3E	-0.53	-35.07	-17.29		
4E	-0.48	-32.60	-14.82		
2OH	0.21	10	.37		
2EOH	0.27	13	.34		
3OH	-0.43	-21.24			
3ЕОН	-0.53	-26.18			
2OH+W	0.21/-0.7	-24	.20		
2EOH+W	0.27/-0.7	-21	.24		

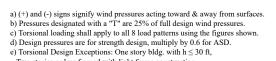
- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10. c) Design building for all wind directions, 4 load patterns per load case.
- d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
- e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof. f) Design pressures are for strength design, multiply by 0.6 for ASD.
- Subject Customer Location Wind Loads 2025D18 This report may not be copied, reproduced or distributed without the written consent of **Engineer Name** ENGINEERING COMPANY INC. Date Street Address City, CA 99999 Page 1/15/2025 ph. (800) 000-0000 www.website.com Copyright © 2025

Load Ca	Load Case B: Longitudinal Direction					
C	CC-f	Design Pre	essure (psf)			
Surface	GCpf	(w/ +GCpi)	(w/ -GCpi)			
1	-0.45	-31.12	-13.34			
2	-0.69	-42.97	-25.19			
3	-0.37	-27.17	-9.38			
4	-0.45	-31.12	-13.34			
5	0.40	10.87	28.65			
6	-0.29	-23.21	-5.43			
1E	-0.48	-32.60	-14.82			
2E	-1.07	-61.74	-43.96			
3E	-0.53	-35.07	-17.29			
4E	-0.48	-32.60	-14.82			
5E	0.61	21.24	39.02			
6E	-0.43	-30.13	-12.35			
2OH	-0.69	-34	08			
2EOH	-1.07	-52	85			
3ОН	-0.37	-18.28				
3ЕОН	-0.53	-26.18				
2EOH+W	-1.07/-0.7	-87.43				
3EOH+W	-0.53/-0.7	-60	.75			

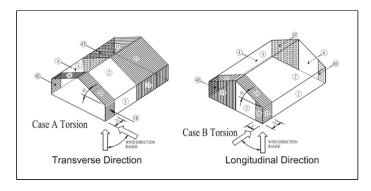


- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
  b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
  c) Design building for all wind directions, 4 load patterns per load case.
  d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
  e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
  f) Design pressures are for strength design, multiply by 0.6 for ASD.

Torsional Load Cases						
Surface	Load Case	GCpf	Design Pressure (psf)			
Surface	Load Case	ССРІ	(w/+GCpi)	(w/ -GCpi)		
1T	A	-	4.69	9.14		
2T	A	-	0.37	4.82		
3T	A	-	-7.53	-3.09		
4T	A	-	-6.79	-2.35		
5T	В	-	2.72	7.16		
6T	В	-	-5.80	-1.36		



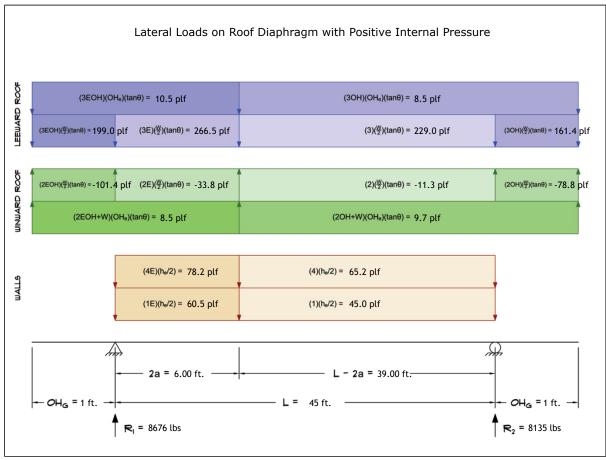
Two stories or less framed with light frame construction, Two stories or less with flexible diaphragms.



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# 5. Wind Load Calculations

#### 1.) <u>Lateral Loads - Transverse Direction</u>:



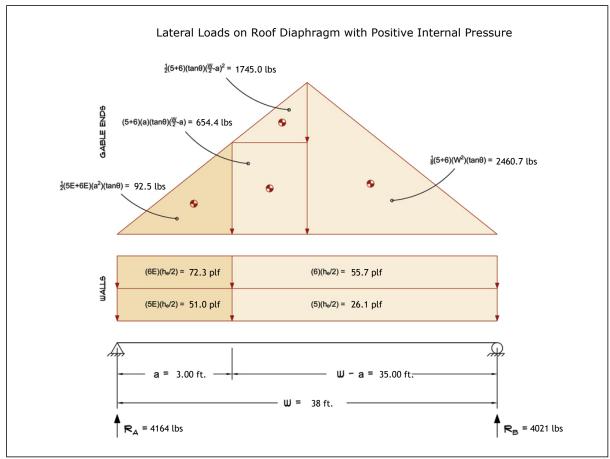
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

Wind Base Shear (ASD)							
Load Case A: Transverse Direction							
Load Case	Load Case Walls (lbs) Roof (lbs) Roof Overhangs (lbs) Total Lateral Load (lbs) R1 (lbs) R2 (lbs)						
Positive Internal Pressure	5132	11172	507	16810	8676	8135	
Negative Internal Pressure	5132	11172	507	16810	8676	8135	
Roof Pressure = 0	5132	0	0	5132	2640	2492	
Min. Pressures (8 psf, 16 psf)	1728	2736	272	4736	2368	2368	

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

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#### 2.) <u>Lateral Loads - Longitudinal Direction</u>:



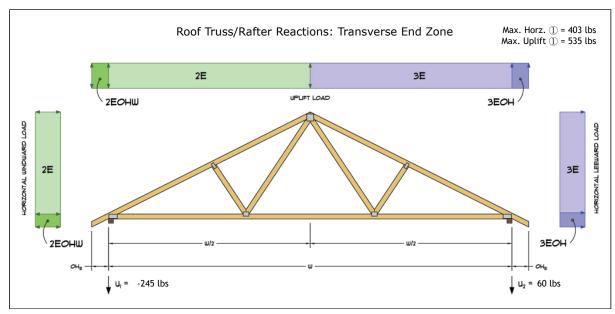
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
  b) Strength design values multiplied by 0.6 to obtain ASD values.
  c) Where the length of building (L) exceeds 4X the mean roof height (h), wind drag forces should additionally be considered.

	Wind Base Shear (ASD)						
	Load Case B: Longitudinal Direction						
Load Case	Load Case Walls (lbs) Gable Ends (lbs) Roof (lbs) Total Lateral Load (lbs) RA (lbs) RB (l						
Positive Internal Pressure	Positive Internal Pressure 3233 4952 0 8185 4164 40						
Negative Internal Pressure	Negative Internal Pressure         3233         4952         0         8185         4164         402						
Roof Pressure = 0	Roof Pressure = 0 3233 4952 0 8185 4164 4021						
Min. Pressures (8 psf, 16 psf)	1459	2310	0	3770	1885	1885	

- a) Bottom half of wall neglected in tributary area calculations.
   b) Strength design values multiplied by 0.6 to obtain ASD values.

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#### 3.) Roof Truss Reactions:



- a) Strength design values multiplied by 0.6 to obtain ASD values.
- b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)						
w/ Positive Internal Pressure						
Load Case Horizontal Load (lbs) Gross Uplift (lbs) Net Uplift (lbs) U1 (lbs) U2						
Transverse Int. Zone	-319	472	-217	-227	11	
Transverse End Zone	-403	503	-185	-245	60	
Longitudinal Int. Zone	169	1108	419	276	143	
Longitudinal End Zone	285	1535	846	535	311	

- a) Gross Uplift calculations do not include any counteracting roof dead loads.
- b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10. c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads. d) Loads based on truss spacing calculated at 16" o/c.

- e) Negative values for horizontal load indicate load acting in windward direction (tranverse load cases).
- f) Negative values for uplift indicate net downward force (zero uplift).

\*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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