

Wind Load Report - Niles St Restaurant

1. Site & Building Data

Roof Type: Gable Wind Speed (ult): 110 mph \mathbf{C} Exposure Category: Enclosed **Enclosure Class:** 38.3 ft. Building Width (W): 48 ft. Building Length (L): Eave Height (he): 14 ft. Foundation Height (hf): 0 ft. Roof Pitch: 2 /12 Eave Overhang (OH_e): 0 ft. Gable Overhang (OHg): 0 ft.

2. Parameters & Coefficients

Topographic Factor (K _{zt}):	1.0	
Directionality Factor (K _d):	.85	
Roof Angle (θ):	9.46	deg.
Mean Roof Height (h):	15.60	ft.
Ridge Height (h _r):	17.19	ft.
Pos. Internal Pressure (+GCpi):	+0.18	
Neg. Internal Pressure (-GCpi):	-0.18	
Velocity Pressure Exp. Coeff. (K _h):	0.86	@ z=h
Velocity Pressure (qh):	22.53	psf
End Zone Width (a):	3.00	ft.

19.15 ft.

3. Design Assumptions and Notes

Code Standard: **ASCE 7-10** Geometry: Regular-Shaped Bldg. Height Class: Low-Rise Building

Notes:

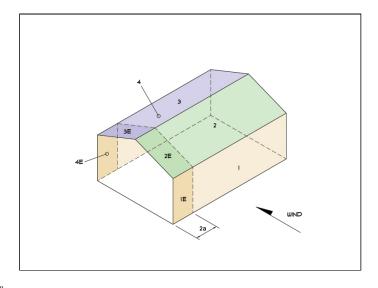
4. Design Loads

Zone 2/2E Dist.:

Top Chord Dead Load: 7 psf Bottom Chord Dead Load: 10 psf Truss/Rafter Spacing: 24 in. o/c

4. Design Wind Pressures: MWFRS Envelope Procedure

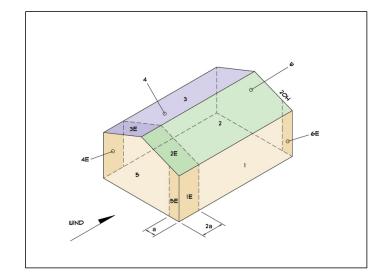
Load Case A: Transverse Direction						
Surface	CCf	Design Pressure (psf)				
Surface	GCpf	(w/ +GCpi)	(w/ -GCpi)			
1	0.44	5.83	13.94			
2	-0.69	-19.61	-11.49			
3	-0.40	-13.13	-5.02			
4	-0.33	-11.53	-3.42			
1E	0.67	10.96	19.08			
2E	-1.07	-28.17	-20.06			
3E	-0.58	-17.07	-8.96			
4E	-0.49	-15.15	-7.04			



- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10. c) Design building for all wind directions, 4 load patterns per load case.
- d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
- e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof. f) Design pressures are for strength design, multiply by 0.6 for ASD.

Subject Wind Loads	Customer	Location 15	13 Niles St Bakersfield		Job No. 40827
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
8/27/2024	Street Address City, CA 99999 ph. (800) 000-0000 www.we	99 vebsite.com	COMPANY LOGO	Copyright © 2024	Page 1

Surface	CCf	Design Pre	ssure (psf)
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)
1	-0.45	-14.20	-6.08
2	-0.69	-19.61	-11.49
3	-0.37	-12.39	-4.28
4	-0.45	-14.20	-6.08
5	0.40	4.96	13.07
6	-0.29	-10.59	-2.48
1E	-0.48	-14.87	-6.76
2E	-1.07	-28.17	-20.06
3E	-0.53	-16.00	-7.89
4E	-0.48	-14.87	-6.76
5E	0.61	9.69	17.80
6E	-0.43	-13.75	-5.63

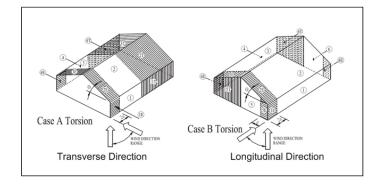


- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
 b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
 c) Design building for all wind directions, 4 load patterns per load case.
 d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
 e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
 f) Design pressures are for strength design, multiply by 0.6 for ASD.

Torsional Load Cases								
Surface	Load Case	GCpf	Design Pressure (psf)					
Surface	Load Case	ССРІ	(w/+GCpi)	(w/ -GCpi)				
1T	A	-	1.46	3.49				
2T	A	-	-4.90	-2.87				
3T	A	-	-3.28	-1.25				
4T	A	-	-2.88	-0.85				
5T	В	-	1.24	3.27				
6T	В	-	-2.65	-0.62				



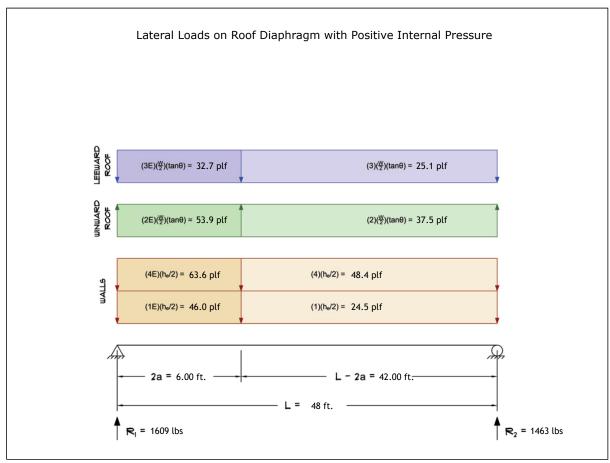
- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) Pressures designated with a "T" are 25% of full design wind pressures. c) Torsional loading shall apply to all 8 load patterns using the figures shown. d) Design pressures are for strength design, multiply by 0.6 for ASD. e) Torsional Design Exceptions: One story bldg, with h \leq 30 ft, Two stories or less framed with light frame construction, Two stories or less with flexible diaphragms.



Subject Wind Loads	Customer	Location 15	13 Niles St Bakersfield		Job No. 40827
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
8/27/2024	Street Address City, CA 999 ph. (800) 000-0000 www.v	99 vebsite.com	COMPANY EGGS		Page 2

5. Wind Load Calculations

1.) <u>Lateral Loads - Transverse Direction</u>:



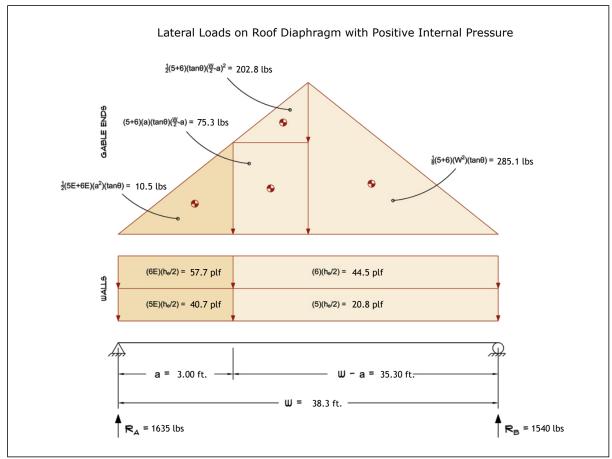
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

Wind Base Shear (ASD)								
Load Case A: Transverse Direction								
Load Case Walls (lbs) Roof (lbs) Roof Overhangs (lbs) Total Lateral Load (lbs) R1 (lbs) R2 (lbs)								
Positive Internal Pressure	3720	-648	0	3072	1609	1463		
Negative Internal Pressure	3720	-648	0	3072	1609	1463		
Roof Pressure = 0	3720	0	0	3720	1957	1764		
Min. Pressures (8 psf, 16 psf)	3226	735	0	3961	1980	1980		

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

Subject Wind Loads	Customer	Location 15	13 Niles St Bakersfield		Job No. 40827
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
8/27/2024	Street Address City, CA 999 ph. (800) 000-0000 www.v	999 vebsite.com	COMPANY LOGO	Copyright © 2024	Page 3

2.) <u>Lateral Loads - Longitudinal Direction</u>:



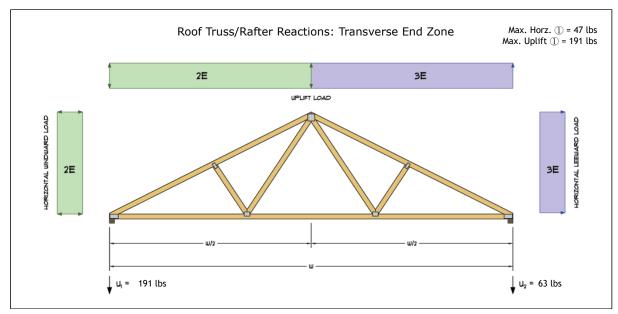
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
 b) Strength design values multiplied by 0.6 to obtain ASD values.
 c) Where the length of building (L) exceeds 4X the mean roof height (h), wind drag forces should additionally be considered.

Wind Base Shear (ASD)								
Load Case B: Longitudinal Direction								
Load Case	Total Lateral Load (lbs)	R _A (lbs)	R _B (lbs)					
Positive Internal Pressure	2601	574	0	3174	1635	1540		
Negative Internal Pressure	2601	574	0	3174	1635	1540		
Roof Pressure = 0	2601	574	0	3174	1635	1540		
Min. Pressures (8 psf, 16 psf)	2574	587	0	3161	1580	1580		

- a) Bottom half of wall neglected in tributary area calculations.
 b) Strength design values multiplied by 0.6 to obtain ASD values.

Subject	Customer	Location			Job No.
Wind Loads		15	513 Niles St Bakersfield		40827
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
8/27/2024	Street Address City, CA 999 ph. (800) 000-0000 www.v	99 vebsite.com	COMPANY LOGO Copyright © 202		Page 4

3.) Roof Truss Reactions:



- a) Strength design values multiplied by 0.6 to obtain ASD values.
- b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)								
w/ Positive Internal Pressure								
Load Case	Horizontal Load (lbs)	Gross Uplift (lbs)	Net Uplift (lbs)	U ₁ (lbs)	U ₂ (lbs)			
Transverse Int. Zone	25	752	-33	20	-54			
Transverse End Zone	42	1040	254	191	63			
Longitudinal Int. Zone	28	735	-50	16	-67			
Longitudinal End Zone	47	1015	229	185	45			

- a) Gross Uplift calculations do not include any counteracting roof dead loads.
- b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10. c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads. d) Loads based on truss spacing calculated at 24" o/c.

- e) Negative values for horizontal load indicate load acting in windward direction (tranverse load cases).
- f) Negative values for uplift indicate net downward force (zero uplift).

*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

Subject Wind Loads	Customer	Location 15	13 Niles St Bakersfield		Job No. 40827
Engineer Name	ENGINEERING CO		STRUCTURAL ENGINEERS	This report may not be copied, reproduced or distributed without the written consent of Engineering Company Inc.	Rev.
8/27/2024	Street Address City, CA 99999 ph. (800) 000-0000 www.wel		COMPANY LOGO	Copyright © 2024	Page 5