

Wind Load Report - New Residence

1. Site & Building Data

Roof Type: Gable Wind Speed (ult): 110 mph \mathbf{C} Exposure Category: Enclosed **Enclosure Class:** 30 ft. Building Width (W): 80 ft. Building Length (L): Eave Height (he): 9 ft. Foundation Height (hf): 0 ft. Roof Pitch: 5 /12 Eave Overhang (OH_e): 2 ft. Gable Overhang (OHg): 2 ft.

2. Parameters & Coefficients

Topographic Factor (Kzt):	1.0	
Directionality Factor (K _d):	.85	
Roof Angle (θ):	22.62	deg.
Mean Roof Height (h):	12.13	ft.
Ridge Height (h _r):	15.25	ft.
Pos. Internal Pressure (+GCpi):	+0.18	
Neg. Internal Pressure (-GCpi):	-0.18	
Velocity Pressure Exp. Coeff. (Kh):	0.85	@ z=h
Velocity Pressure (qh):	22.35	psf
End Zone Width (a):	3.00	ft.

15.00 ft.

3. Design Assumptions and Notes

Code Standard: **ASCE 7-10** Geometry: Regular-Shaped Bldg. Height Class: Low-Rise Building

Notes:

4. Design Loads

Zone 2/2E Dist.:

Top Chord Dead Load: 7 psf Bottom Chord Dead Load: 10 psf Truss/Rafter Spacing: 24 in. o/c

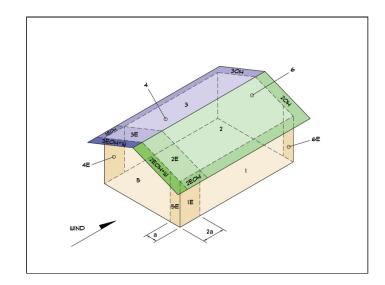
4. Design Wind Pressures: MWFRS Envelope Procedure

Load Case A: Transverse Direction					
Surface	CC-f	Design Pressure (psf)			
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)		
1	0.54	8.00	16.04		
2	-0.45	-14.18	-6.13		
3	-0.47	-14.46	-6.41		
4	-0.41	-13.28	-5.24		
1E	0.77	13.21	21.26		
2E	-0.72	-20.09	-12.05		
3E	-0.65	-18.51	-10.46		
4E	-0.60	-17.39	-9.34		
2OH	-0.45	-10	.15		
2EOH	-0.72	-16	.07		
3OH	-0.47	-10	.44		
3ЕОН	-0.65	-14.49			
2OH+W	-0.45/-0.7	-25.80			
2EOH+W	-0.72/-0.7	-31	.71		

- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10. c) Design building for all wind directions, 4 load patterns per load case.
- d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
- e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof. f) Design pressures are for strength design, multiply by 0.6 for ASD.

Subject Wind Loads	Customer	Location 2121	0 Inyo St. Lost Hills, CA		Job No. 40808
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Load Case B: Longitudinal Direction						
Surface	CC-f	Design Pressure (psf)				
Surface	GCpf	(w/ +GCpi)	(w/ -GCpi)			
1	-0.45	-14.08	-6.03			
2	-0.69	-19.45	-11.40			
3	-0.37	-12.29	-4.25			
4	-0.45	-14.08	-6.03			
5	0.40	4.92	12.96			
6	-0.29	-10.50	-2.46			
1E	-0.48	-14.75	-6.71			
2E	-1.07	-27.94	-19.89			
3E	-0.53	-15.87	-7.82			
4E	-0.48	-14.75	-6.71			
5E	0.61	9.61	17.66			
6E	-0.43	-13.63	-5.59			
2OH	-0.69	-15	.42			
2EOH	-1.07	-23	.92			
3ОН	-0.37	-8.27				
3ЕОН	-0.53	-11.85				
2EOH+W	-1.07/-0.7	-39.56				
3EOH+W	-0.53/-0.7	-27	.49			

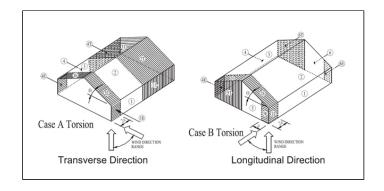


- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
 b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10.
 c) Design building for all wind directions, 4 load patterns per load case.
 d) Total horizontal shear shall not be less than that by neglecting roof wind forces.
 e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
 f) Design pressures are for strength design, multiply by 0.6 for ASD.

Torsional Load Cases							
Surface	Load Case	GCpf	Design Pressure (p				
Surface	Load Case	ССРІ	(w/+GCpi)	(w/-GCpi)			
1T	A	-	2.00	4.01			
2T	A	-	-3.54	-1.53			
3T	A	-	-3.61	-1.60			
4T	A	-	-3.32	-1.31			
5T	В	-	1.23	3.24			
6T	В	-	-2.63	-0.61			

- a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) Pressures designated with a "T" are 25% of full design wind pressures. c) Torsional loading shall apply to all 8 load patterns using the figures shown. d) Design pressures are for strength design, multiply by 0.6 for ASD. e) Torsional Design Exceptions. One story bldg. with $h \leq 30 \, \text{ft},$

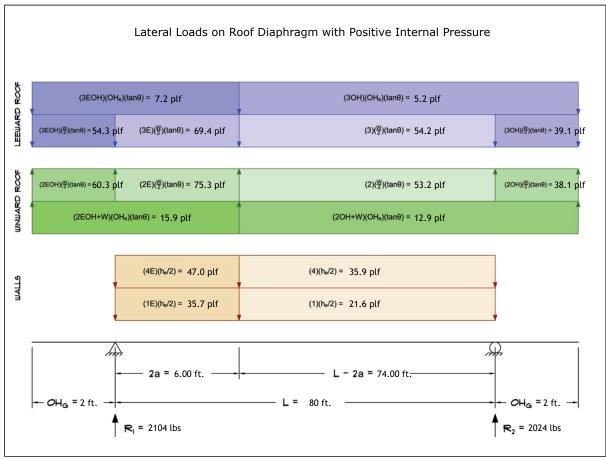
Two stories or less framed with light frame construction, Two stories or less with flexible diaphragms.



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5. Wind Load Calculations

1.) <u>Lateral Loads - Transverse Direction</u>:



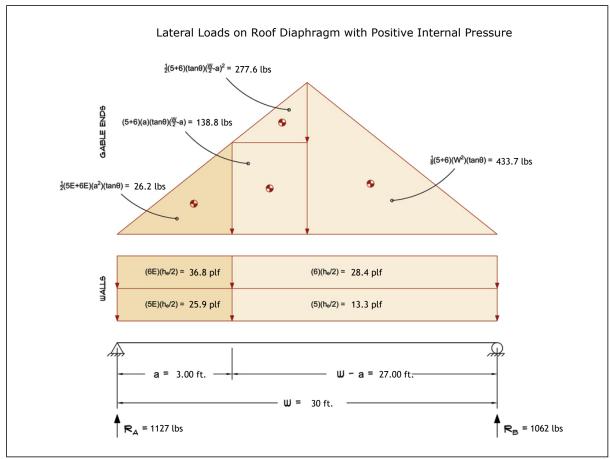
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

	Wind Base Shear (ASD)							
	Load Case A: Transverse Direction							
Load Case	Load Case Walls (lbs) Roof (lbs) Roof Overhangs (lbs) Total Lateral Load (lbs) R1 (lbs) R2 (lbs)							
Positive Internal Pressure	4748	43	-662	4128	2104	2024		
Negative Internal Pressure	4748	43	-662	4128	2104	2024		
Roof Pressure = 0	4748	0	0	4748	2444	2304		
Min. Pressures (8 psf, 16 psf)	3456	2400	456	6312	3156	3156		

- a) Bottom half of wall neglected in tributary area calculations.
- b) Strength design values multiplied by 0.6 to obtain ASD values.

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2.) <u>Lateral Loads - Longitudinal Direction</u>:



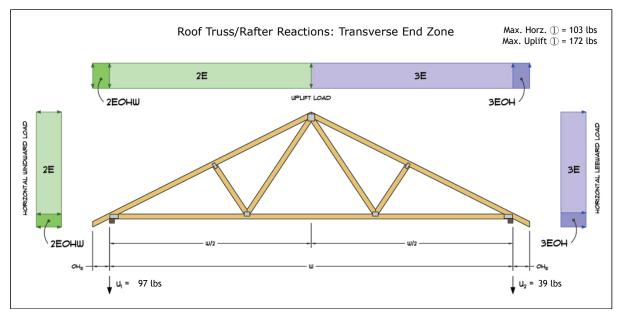
- a) (-) signs signify wind lateral forces acting opposite to the direction of the arrows shown.
 b) Strength design values multiplied by 0.6 to obtain ASD values.
 c) Where the length of building (L) exceeds 4X the mean roof height (h), wind drag forces should additionally be considered.

	Wind Base Shear (ASD)							
	Load Case B: Longitudinal Direction							
Load Case	Walls (lbs)	Gable Ends (lbs)	Roof (lbs)	Total Lateral Load (lbs)	R _A (lbs)	R _B (lbs)		
Positive Internal Pressure	1313	876	0	2189	1127	1062		
Negative Internal Pressure	1313	876	0	2189	1127	1062		
Roof Pressure = 0	1313	876	0	2189	1127	1062		
Min. Pressures (8 psf, 16 psf)	1296	900	0	2196	1098	1098		

- a) Bottom half of wall neglected in tributary area calculations.
 b) Strength design values multiplied by 0.6 to obtain ASD values.

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3.) Roof Truss Reactions:



- a) Strength design values multiplied by 0.6 to obtain ASD values.
- b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)							
w/ Positive Internal Pressure							
Load Case	Horizontal Load (lbs)	Gross Uplift (lbs)	Net Uplift (lbs)	U ₁ (lbs)	U ₂ (lbs)		
Transverse Int. Zone	13	602	-67	-15	-52		
Transverse End Zone	29	806	136	97	39		
Longitudinal Int. Zone	61	628	-41	21	-62		
Longitudinal End Zone	103	874	205	172	33		

- a) Gross Uplift calculations do not include any counteracting roof dead loads.
- b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10. c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads. d) Loads based on truss spacing calculated at 24" o/c.
- e) Negative values for horizontal load indicate load acting in windward direction (tranverse load cases).
- f) Negative values for uplift indicate net downward force (zero uplift).

*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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