# Wind Load Report

### 1. Site & Building Data

Roof Type:	Gable
Wind Speed (ult):	115 mph
Exposure Category:	С
Enclosure Class:	Enclosed
Building Width (W):	30 ft.
Building Length (L):	48 ft.
Eave Height (he):	10 ft.
Foundation Height (hf):	0 ft.
Roof Pitch:	4 /12
Eave Overhang (OH <sub>e</sub> ):	1 ft.
Gable Overhang (OHg):	1 ft.

## 2. Parameters & Coefficients

Topographic Factor (K <sub>zt</sub> ):	1.0
Directionality Factor (Kd):	.85
Roof Angle ( $\theta$ ):	18.43 deg.
Mean Roof Height (h):	12.50 ft.
Ridge Height (h <sub>r</sub> ):	15.00 ft.
Pos. Internal Pressure (+GCpi):	+0.18
Neg. Internal Pressure (-GCpi):	-0.18
Velocity Pressure Exp. Coeff. (Kh):	0.85 @ z=h
Velocity Pressure (qh):	24.43 psf
End Zone Width (a):	3.00 ft.
Zone 2/2E Dist.:	15.00 ft.

#### 3. Design Assumptions and Notes 4. Design Loads

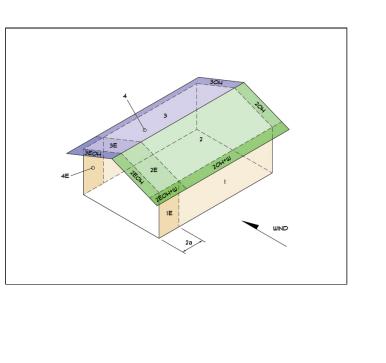
Code Standard:	ASCE 7-10
Geometry:	Regular-Shaped Bldg.
Height Class:	Low-Rise Building
Notes:	

Top Chord Dead Load:	7 psf
Bottom Chord Dead Load:	10 psf
Truss/Rafter Spacing:	24 in. o/c

## 4. Design Wind Pressures: MWFRS Envelope Procedure

Load C	ase A: Tra	ansverse D	irection	
Sunfran	CCaf	Design Pre	essure (psf)	
Surface	GCpf	(w/+GCpi)	(w/ -GCpi)	
1	0.52	8.22	17.01	
2	-0.69	-21.25	-12.46	
3	-0.47	-15.84	-7.05	
4	-0.42	-14.54	-5.75	
1E	0.78	14.66	23.46	
2E	-1.07	-30.54	-21.74	
3E	-0.67	-20.85	-12.05	
4E	-0.62	-19.50	-10.70	
2OH	-0.69	-16	5.86	
2EOH	-1.07	-26	5.14	
3OH	-0.47	-11.45		
3EOH	-0.67	-16.45		
2OH+W	-0.69/-0.7	-33.96		
2EOH+W	-1.07/-0.7	-43	5.24	

a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7-10. c) Design building for all wind directions, 4 load patterns per load case. a) Total horizontal shear shall not be less than that by neglecting roof wind forces.
b) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof.
f) Design pressures are for strength design, multiply by 0.6 for ASD.



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Wind Loads					2024D229
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Surface	GCpf	Gent				Design Pressure (psf)	u /
	î	(w/+GCpi)	(w/ -GCpi)				
1	-0.45	-15.39	-6.60				
2	-0.69	-21.25	-12.46				
3	-0.37	-13.44	-4.64				
4	-0.45	-15.39	-6.60				
5	0.40	5.37	14.17				
6	-0.29	-11.48	-2.69				
1E	-0.48	-16.12	-7.33				
2E	-1.07	-30.54	-21.74				
3E	-0.53	-17.34	-8.55				
4E	-0.48	-16.12	-7.33				
5E	0.61	10.50	19.30				
6E	-0.43	-14.90	-6.11				
2OH	-0.69	-16	.86				
2EOH	-1.07	-26	.14				
3OH	-0.37	-9.	04				
3EOH	-0.53	-12	.95				
2EOH+W	-1.07/-0.7	-43	.24				
3EOH+W	-0.53/-0.7	-30.05					
a) (+) and (-) signs signify wind pressures acting toward & away from surfaces. b) External Pressure Coefficients linearly interpolated from Fig. 28.4-1 ASCE 7 c) Design building for all wind directions, 4 load patterns per load case. d) Total horizontal shear shall not be less than that by neglecting roof wind force							

3E 1E

**Torsional Load Cases** Design Pressure (psf) Surface Load Case GCpf (w/+GCpi) (w/-GCpi) 1T A 2.05 4.25 -5.31 -3.11 2T A \_ 3T -3.96 -1.76 А -4T A --3.64 -1.44

e) Min. wind load for enclosed or partially enclosed bldg.: 16 psf wall, 8 psf roof. f) Design pressures are for strength design, multiply by 0.6 for ASD.

Case A Torsion Case B Torsion Longitudinal Direction Transverse Direction

 a) (+) and (-) signs signify wind pressures acting toward & away from surfaces.
 b) Pressures designated with a "T" are 25% of full design wind pressures. c) Torsional loading shall apply to all 8 load patterns using the figures shown. d) Design pressures are for strength design, multiply by 0.6 for ASD. e) Torsional Design Exceptions: One story bldg. with  $h \le 30$  ft,

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1.34

-2.87

3.54

-0.67

Two stories or less framed with light frame construction, Two stories or less with flexible diaphragms.

В

В

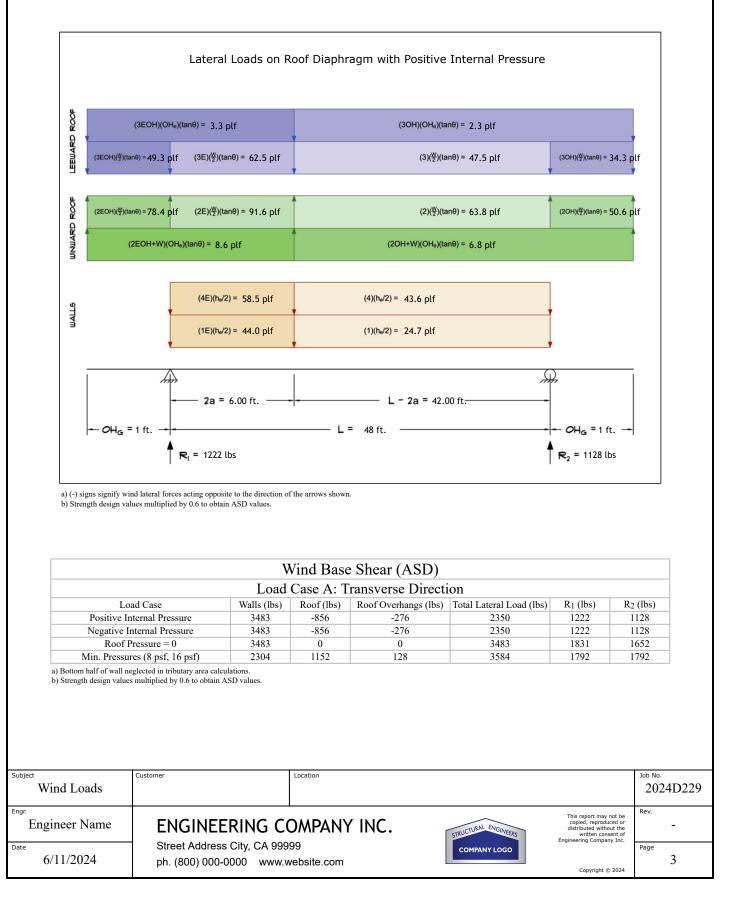
5T

6T

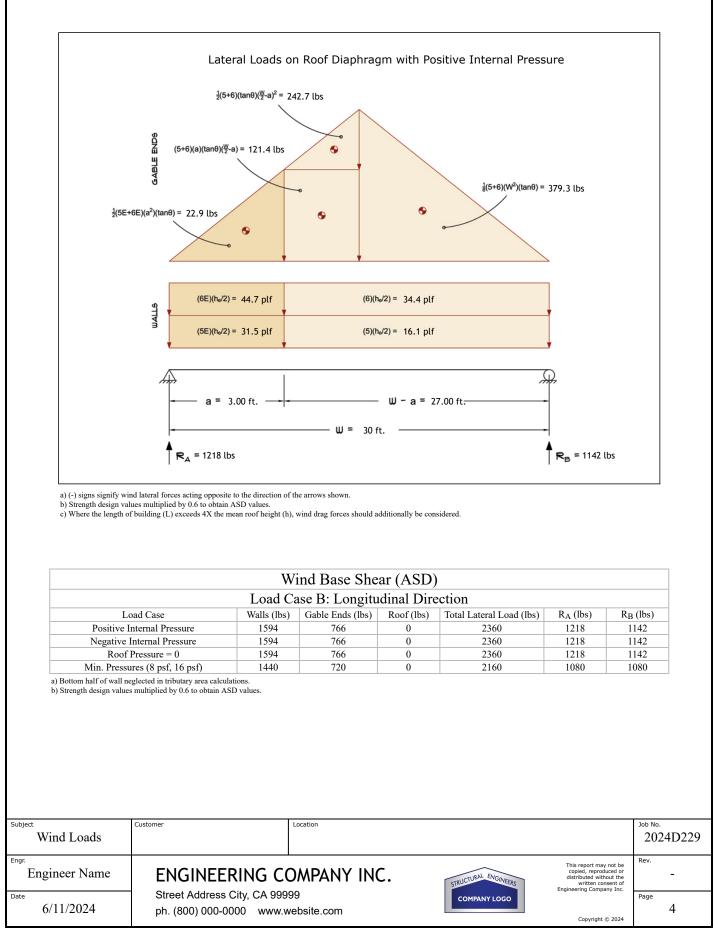
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## 5. Wind Load Calculations

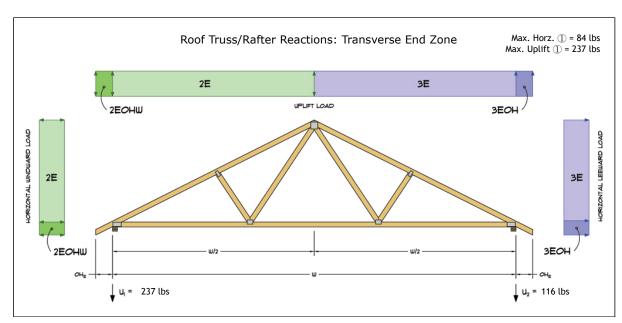
#### 1.) Lateral Loads - Transverse Direction:



#### 2.) Lateral Loads - Longitudinal Direction:



#### 3.) Roof Truss Reactions:



a) Strength design values multiplied by 0.6 to obtain ASD values.

b) Windward loads may be positive or negative depending on pitch of roof.

Roof Truss/Rafter Reactions (ASD)					
w/ Positive Internal Pressure					
Load Case Horizontal Load (lbs) Gross Uplift (lbs) Net Uplift (lbs) U1 (lbs) U2 (lbs)					
Transverse Int. Zone	41	722	79	78	1
Transverse End Zone	69	996	353	237	116
Longitudinal Int. Zone	50	655	12	46	-34
Longitudinal End Zone	84	909	265	200	65

a) Gross Uplift calculations do not include any counteracting roof dead loads.

b) Net Uplift calculations include counteracting roof dead loads multiplied by 0.6 per load case (7) ASCE 7-10.
 c) Strength design values multiplied by 0.6 to obtain ASD values for wind loads.
 d) Loads based on truss spacing calculated at 24" o/c.

e) Negative values for horizontal load indicate load acting in windward direction (tranverse load cases).

f) Negative values for uplift indicate net downward force (zero uplift).

\*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The wind load calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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