

Beam Design - Res Collins-Drake

1. Beam Data

2. Design Loads

82

165

287.9 lbs

292.7 lbs

plf

plf

Load Type: Uniform Dist. Load Live Load: Support: Simple Beam Dead Load: Glulam Selfweight: Beam Type: Species: Western Species Dist. Selfweight: 14.42 plf Grade: 24F-V5 1.7E DF/HF Total Weight: Size: 5.125 x 12

Design Span (L): 19.97 ft. 19.63 ft. Clear Span: 20.30 ft. Total Span: 4 in. Bearing (lb): Quantity (N): 1

4. Design Assumptions and Notes

Code Standard: IBC 2015, NDS 2015 Lateral Support: braced Defl. Limits: 180|120 Bending Stress: Parallel to Grain

Load Duration: 1.00 Notes:

Exposure: dry $T \le 100$ °F Temperature: Orientation: Vertical

3. Design Options

5. Adjustment Factors

Factor	Description	Fb	Ft	$F_{\mathbf{v}}$	Fc	Fc⊥	E/E _{min}
C_{D}	Load Duration Factor	1.00	1.00	1.00	1.00	-	-
$C_{\mathbf{M}}$	Wet Service Factor	1	1	1	1	1	1
Ct	Temperature Factor	1	1	1	1	1	1
C_{L}	Beam Stability Factor	1	-	-	-	_	-
Cv	Volume Factor	1.0 ^b	-	-	-	-	_
Cfu	Flat Use Factor	N/A ^c	-	-	-	-	-

- a) Adjustment factors per AWC NDS 2015 and NDS 2015 Supplement.
- b) The volume factor, C_V, shall not apply simultaneously with the beam stability factor, C_L. The lesser factor shall apply.
- c) Only applies when glulam beam is loaded in bending about the y-y axis.

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6. Beam Calculations

Determine reference design values, sectional properties and self weight of beam:

 $A = b \times d$

,

,

where:

b = Breadth of rectangular beam in bending (in.)

d = Depth of rectangular beam in bending (in.)

A = Cross sectional area of beam (in.²)

 S_X = Section modulus about the X-X axis (in.³)

 S_y = Section modulus about the Y-Y axis (in.³)

 $I_X = Moment of inertia about the X-X axis (in.⁴)$

 $I_y = Moment of inertia about the Y-Y axis (in.⁴)$

b = 5.125 in.

d = 12.000 in.

 $A = 5.125 \times 12.000 = 61.50 \text{ in.}^2$

 $S_x = (5.125)(12.000)^2/6 = 123.00 \text{ in.}^3$

$$S_V = (5.125)^2 (12.000)/6 = 52.53 \text{ in.}^3$$

$$I_x = (5.125)(12.000)^3/12 = 738.00 \text{ in.}^4$$

$$I_y = (5.125)^3 (12.000)/12 = 134.61 \text{ in.}^4$$

Reference Design Values from Table 5A NDS Supplement (Reference Design Values for Structural Glue Laminated Softwood Timber Combinations).

Species & Grade	F _{bx} +	F _{bx} -	$F_{c\perp x}$	Fvx	Ex	Eminx	Fby	F _{c⊥y}	Fvy	Ey	Eminy	Ft	Fc	G
24F-V5 1.7E DF/HF	2400	1600	650	215	1700000	900000	1350	375	200	1500000	790000	1100	1450	0.5

The following formula shall be used to determine the density of wood (lbs/ft³. (NDS Supplement Sec. 3.1.3)

where:

 $\rho_{\rm W}$ = Density of wood (lbs/ft³

G = Specific gravity of wood (dimensionless)

m.c. = Moisture content of wood (percentile)

G = 0.5

m.c. = 16 % (Max. moisture content at dry service conditions)

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$$= 33.76 \text{ lbs/ft}^3$$

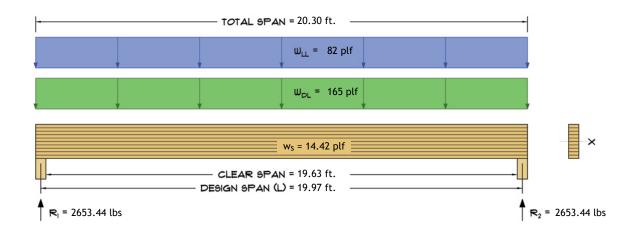
$$\begin{aligned} & Volume_{total} = N[A \ x \ (L+l_b)] = 1 \ x \ [61.50 \ x \ (239.60+4)] \ x \ (12 \ in./ft.)^3 = 8.67 \ ft^3 \\ & Volume_{span} = N[A \ x \ L] = 1 \ x \ [61.50 \ x \ 239.60] \ x \ (12 \ in./ft.)^3 = 8.53 \ ft^3 \end{aligned}$$

Total Weight (W_T) = ρ_W x Volume_{total} = 33.76 x 8.67 = 292.7 lbs Self Weight (W_S) = ρ_W x Volume_{span} = 33.76 x 8.53 = 287.9 lbs

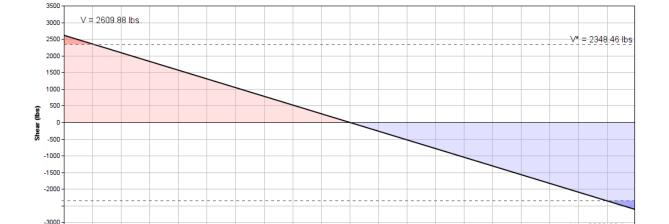
Distributed Self Weight (w_s) = = 14.42 plf

Shear Equation: V(x) = -21.78x + 2609.9

Load, Shear and Moment Diagrams:



Beam - Shear Diagram



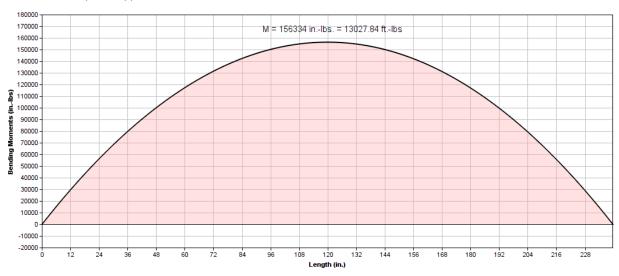
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Beam - Moment Diagram

Moment Equation: $M(x) = -10.89x^2 + 2609.9x$



1.) Bending:

Members subject to bending stresses shall be proportioned so that the actual bending stress or moment shall not exceed the adjusted bending design value:

$$f_b \leq F_{b} \text{'} \ \textit{(NDS Sec. 3.3.1)}$$

where:

$$f_b = M / S$$

$$F_{bx}{'} = F_{bx}(C_D)(C_M)(C_t)(C_V) \quad \text{or} \quad F_{bx}{'} = F_{bx}(C_D)(C_M)(C_t)(C_L)$$

Beam is braced laterally along its compression edge. Laterial stability is not a consideration:

$$C_L$$
 = Beam Stability Factor = 1.0

$$C_{V} = = 1.0$$

Neither volume effect nor lateral stability govern:

$$C_{V} = 1.0$$

$$F_{bx}' = (2400)(1.00)(1)(1)(1.0) = 2400.0 \text{ psi}$$

$$f_b = = 1271.0 \ psi$$

$$f_b = 1271.0 \text{ psi} < F_{bx'} = 2400.0 \text{ psi} \text{ (CSI} = 0.53)$$
 ? **OK**

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2.) Shear:

Members subject to shear stresses shall be proportioned so that the actual shear stress parallel to grain or shear force at any cross section of the bending member shall not exceed the adjusted shear design value:

$$f_v \le F_{v'}$$
 (NDS Sec. 3.4.1)

where:

$$f_v =$$

$$F_v' = F_v(C_D)(C_M)(C_t)$$

$$F_{vx'} = (215)(1.00)(1)(1) = 215.00 \text{ psi}$$

Shear Reduction: For beams supported by full bearing on one surface and loads applied to the opposite surface, uniformly distributed loads within a distance, d, from supports equal to the depth of the bending member shall be pemitted to be ignored. For beams supported by full bearing on one surface and loads applied to the opposite surface, concentrated loads within a distance equal to the depth of the bending member from supports shall be permitted to be multiplied by x/d where x is the distance from the beam support face to the load. See NDS 2015, Figure 3C.

$$f_{v}* = 57.28 \text{ psi}$$

$$f_v^* = 57.28 \text{ psi} < F_{vx'} = 215.00 \text{ psi} \text{ (CSI} = 0.27) ? OK$$

No Reduction in Shear (conservative):

$$f_v = = 63.66 \text{ psi}$$

$$f_v = 63.66 \text{ psi} < F_{vx'} = 215.00 \text{ psi} \text{ (CSI} = 0.30)$$
 ? **OK**

3.) Deflection:

Bending deflections calculated per standard method of engineering mechanics for live load and total load:

LL Allowable: L/180 TL Allowable: L/120

$$E_{x'} = E_{x}(C_{M})(C_{t}) = 1700000(1)(1) = 1700000 \text{ psi}$$

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$$\Delta_{LL} = = 0.23$$
 in.

$$(L/d)_{LL} = 239.60 / 0.23 = 1025$$

$$\Delta_{LL} = 0.23 \text{ in} = L/1025 < L/180$$
 ? **OK**

$$\Delta_{TL} = 0.75 \text{ in.}$$

$$(L/d)_{TL} = 239.60 / 0.75 = 322$$

$$\Delta_{TL} = 0.75 \text{ in} = L/322 < L/120$$
 ? **OK**

4.) Bearing:

Members subject to bearing stresses perpendicular to the grain shall be proportioned so that the actual compressive stress perpendicular to grain shall be based on the net bearing area and shall not exceed the adjusted compression design value perpendicular to grain:

$$f_{c\perp} \leq F_{c\perp}$$
' (NDS Sec. 3.10.2)

where:

$$f_{c\perp} =$$

$$F_{c\perp}' = F_{c\perp}(C_M)(C_t)$$

$$F_{c \perp x'} = (650)(1)(1) = 650.00 \text{ psi}$$

$$A_b = b \times l_b = 5.125 \times 4 = 20.50 \text{ in}^2$$

$$f_{c\perp}$$
 = = 129.4 psi

$$f_{c\perp} = 129.4 \text{ psi} < F_{c\perp x'} = 650.00 \text{ psi} \text{ (CSI} = 0.20) ? OK$$

*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The beam calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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