

# Beam Design

### 1. Beam Data

## 2. Design Loads

Load Type: Uniform Dist. Load Support: Simple Beam Glulam Beam Type: Species: Western Species Grade: 24F-V8 1.8E DF/DF Size: 12.25 x 60 Design Span (L): 84.00 ft. 83.00 ft. Clear Span: 85.00 ft. Total Span: 12 in. Bearing (lb): 2 Quantity (N):

Live Load:1240plfDead Load:200plfSelfweight:28950.2lbsDist. Selfweight:344.65plfTotal Weight:29294.9lbs

## 3. Design Options

## 4. Design Assumptions and Notes

Lateral Support:bracedDefl. Limits:360|240Load Duration:1.15Exposure:dryTemperature: $T \le 100^{\circ}F$ Orientation:Vertical

Code Standard: IBC 2015, NDS 2015 Bending Stress: Parallel to Grain

Notes:

## 5. Adjustment Factors

Factor	Description	Fb	Ft	F <sub>v</sub>	Fc	F <sub>c</sub> ⊥	E/E <sub>min</sub>
CD	Load Duration Factor	1.15	1.15	1.15	1.15	-	-
C <sub>M</sub>	Wet Service Factor	1	1	1	1	1	1
Ct	Temperature Factor	1	1	1	1	1	1
$C_{L}$	Beam Stability Factor	1	_	-	-	-	-
Cv	Volume Factor	0.679 <sup>b</sup>	_	-	-	-	-
Cfu	Flat Use Factor	N/A <sup>c</sup>	_	-	-	-	-

- a) Adjustment factors per AWC NDS 2015 and NDS 2015 Supplement.
- b) The volume factor, C<sub>V</sub>, shall not apply simultaneously with the beam stability factor, C<sub>L</sub>. The lesser factor shall apply.
- c) Only applies when glulam beam is loaded in bending about the y-y axis.

Subject	Customer	Location	Job No.	
Beam Design	Randy Ludwar 736 Main Street, Box 614			
N. Wilkerson	MEDEEK ENGINE	written consent or	Rev.	
6/29/2022	3050 State Route 109 Copa ph. (425) 741-5555 www.r	alis Beach, WA 98535	Page 1	

## 6. Beam Calculations

Determine reference design values, sectional properties and self weight of beam:

 $A = b \times d$ 

,

,

#### where:

b = Breadth of rectangular beam in bending (in.)

d = Depth of rectangular beam in bending (in.)

A = Cross sectional area of beam (in.<sup>2</sup>)

 $S_x$  = Section modulus about the X-X axis (in.<sup>3</sup>)

 $S_y$  = Section modulus about the Y-Y axis (in.<sup>3</sup>)

 $I_X$  = Moment of inertia about the X-X axis (in.  $^4$ )

 $I_y = Moment of inertia about the Y-Y axis (in.<sup>4</sup>)$ 

b = 12.250 in.

d = 60.000 in.

 $A = 12.250 \text{ x } 60.000 = 735.00 \text{ in.}^2$ 

 $S_x = (12.250)(60.000)^2/6 = 7350.00 \text{ in.}^3$ 

 $S_V = (12.250)^2 (60.000)/6 = 1500.63 \text{ in.}^3$ 

 $I_x = (12.250)(60.000)^3/12 = 220500.00 \text{ in.}^4$ 

 $I_y = (12.250)^3 (60.000)/12 = 9191.33 \text{ in.}^4$ 

Reference Design Values from Table 5A NDS Supplement (Reference Design Values for Structural Glue Laminated Softwood Timber Combinations).

Species & Grade	F <sub>bx</sub> +	F <sub>bx</sub> -	$F_{c\perp x}$	F <sub>vx</sub>	Ex	Eminx	Fby	F <sub>c⊥y</sub>	Fvy	Ey	Eminy	Ft	Fc	G
24F-V8 1.8E DF/DF	2400	2400	650	265	1800000	950000	1550	560	230	1600000	850000	1100	1650	0.5

The following formula shall be used to determine the density of wood (lbs/ft<sup>3</sup>. (NDS Supplement Sec. 3.1.3)

#### where:

 $\rho_{\rm W}$  = Density of wood (lbs/ft<sup>3</sup>

G = Specific gravity of wood (dimensionless)

m.c. = Moisture content of wood (percentile)

G = 0.5

m.c. = 16 % (Max. moisture content at dry service conditions)

Beam Design	Randy Ludwar 736 Main Street, Box 614				Geno
N. Wilkerson	MEDEEK ENGINE			This report may not be copied, reproduced or distributed without the written consent of Medeek Engineering Inc.	Rev.
Date 6/29/2022	3050 State Route 109 Copa ph. (425) 741-5555 www.r	lis Beach, WA 98535 medeek.com		Copyright © 2014	Page 2

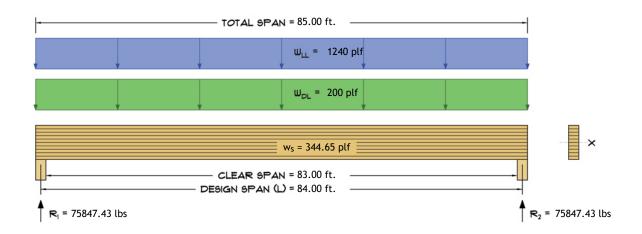
$$= 33.76 \text{ lbs/ft}^3$$

$$\begin{aligned} & Volume_{total} = N[A \text{ x } (L + l_b)] = 2 \text{ x } [735.00 \text{ x } (1008.00 + 12)] \text{ x } (12 \text{ in./ft.})^3 = 867.71 \text{ ft}^3 \\ & Volume_{span} = N[A \text{ x } L] = 2 \text{ x } [735.00 \text{ x } 1008.00] \text{ x } (12 \text{ in./ft.})^3 = 857.50 \text{ ft}^3 \end{aligned}$$

Total Weight (W<sub>T</sub>) = 
$$\rho_W$$
 x Volume<sub>total</sub> = 33.76 x 867.71 = 29294.9 lbs Self Weight (W<sub>S</sub>) =  $\rho_W$  x Volume<sub>span</sub> = 33.76 x 857.50 = 28950.2 lbs

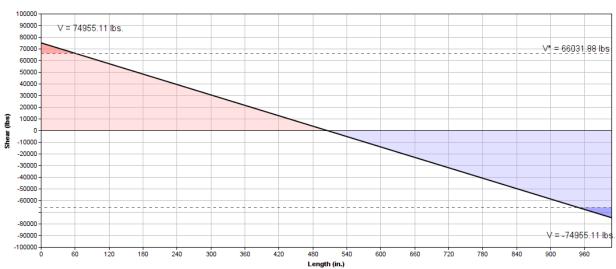
Distributed Self Weight ( $w_s$ ) = = 344.65 plf

### Load, Shear and Moment Diagrams:



Beam - Shear Diagram

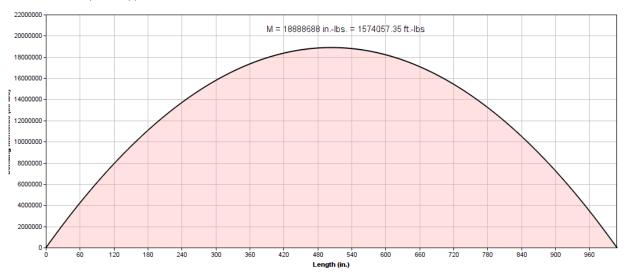
Shear Equation: V(x) = -148.72x + 74955.1



Subject	Customer	Location			Job No.
Beam Design	Randy Ludwar	736 Ma	nin Street, Box 614		Geno
N. Wilkerson	MEDEEK ENGINE			This report may not be copied, reproduced or distributed without the written consent of Medeek Engineering Inc.	Rev.
6/29/2022	3050 State Route 109 Copa ph. (425) 741-5555 www.r			Copyright © 2014	Page 3

#### Beam - Moment Diagram

Moment Equation:  $M(x) = -74.36x^2 + 74955.1x$ 



## 1.) Bending:

Members subject to bending stresses shall be proportioned so that the actual bending stress or moment shall not exceed the adjusted bending design value:

$$f_b \leq F_{b} \text{'} \ \textit{(NDS Sec. 3.3.1)}$$

where:

$$f_b = M / S$$

$$F_{bx}{'} = F_{bx}(C_D)(C_M)(C_t)(C_V) \quad \text{or} \quad F_{bx}{'} = F_{bx}(C_D)(C_M)(C_t)(C_L)$$

Beam is braced laterally along its compression edge. Laterial stability is not a consideration:

 $C_L$  = Beam Stability Factor = 1.0

$$C_V = -0.679$$

Volume effect governs over lateral stability:

$$C_V = 0.679 < C_L = 1$$

$$F_{bx}' = (2400)(1.15)(1)(1)(0.679) = 1874.8 \text{ psi}$$

$$f_b = = 1284.9 \ psi$$

$$f_b = 1284.9 \text{ psi} < F_{bx'} = 1874.8 \text{ psi} \text{ (CSI} = 0.69) ? OK$$

Subject	Customer	Location	Job No.	
Beam Design	Beam Design Randy Ludwar 736 Main Street, Box 614			
N. Wilkerson	MEDEEK ENGINE	written consent	or ne of	
Date 6/29/2022	3050 State Route 109 Copa ph. (425) 741-5555 www.r	llis Beach, WA 98535	Page 4	

### 2.) Shear:

Members subject to shear stresses shall be proportioned so that the actual shear stress parallel to grain or shear force at any cross section of the bending member shall not exceed the adjusted shear design value:

$$f_v \le F_{v'}$$
 (NDS Sec. 3.4.1)

where:

$$f_v =$$

$$F_v' = F_v(C_D)(C_M)(C_t)$$

$$F_{vx'} = (265)(1.15)(1)(1) = 304.75 \text{ psi}$$

Shear Reduction: For beams supported by full bearing on one surface and loads applied to the opposite surface, uniformly distributed loads within a distance, d, from supports equal to the depth of the bending member shall be pemitted to be ignored. For beams supported by full bearing on one surface and loads applied to the opposite surface, concentrated loads within a distance equal to the depth of the bending member from supports shall be permitted to be multiplied by x/d where x is the distance from the beam support face to the load. See NDS 2015, Figure 3C.

$$f_{v}* = 67.38 \text{ psi}$$

$$f_v^* = 67.38 \text{ psi} < F_{vx'} = 304.75 \text{ psi} \text{ (CSI} = 0.22) ? OK$$

No Reduction in Shear (conservative):

$$f_v = = 76.48 \text{ psi}$$

$$f_v = 76.48 \text{ psi} < F_{vx'} = 304.75 \text{ psi} \text{ (CSI} = 0.25)$$
 ? **OK**

### 3.) Deflection:

Bending deflections calculated per standard method of engineering mechanics for live load and total load:

LL Allowable: L/360 TL Allowable: L/240

$$E_{x'} = E_{x}(C_{M})(C_{t}) = 1800000(1)(1) = 18000000 \text{ psi}$$

Subject	Customer	Location	Job No.		
Beam Design	Beam Design Randy Ludwar 736 Main Street, Box 614				
N. Wilkerson	MEDEEK ENGINE	written Medeek Engin	roduced or vithout the consent of		
6/29/2022	3050 State Route 109 Copa ph. (425) 741-5555 www.r	ilis Beach, WA 98535 medeek.com	Page 5		

$$\Delta_{LL} = = 1.75$$
 in.

$$(L/d)_{LL} = 1008.00 / 1.75 = 576$$

$$\Delta_{LL} = 1.75 \text{ in} = L/576 < L/360 ? OK$$

$$\Delta_{TL} = 2.52 \text{ in.}$$

$$(L/d)_{TL} = 1008.00 / 2.52 = 400$$

$$\Delta_{TL} = 2.52 \text{ in} = L/400 < L/240$$
 ? **OK**

### 4.) Bearing:

Members subject to bearing stresses perpendicular to the grain shall be proportioned so that the actual compressive stress perpendicular to grain shall be based on the net bearing area and shall not exceed the adjusted compression design value perpendicular to grain:

$$f_{c\perp} \leq F_{c\perp}$$
' (NDS Sec. 3.10.2)

where:

$$f_{c\perp} =$$

$$F_{c\perp}' = F_{c\perp}(C_M)(C_t)$$

$$F_{c \perp x'} = (650)(1)(1) = 650.00 \text{ psi}$$

$$A_b = b \times l_b = 12.25 \times 12 = 147.00 \text{ in}^2$$

$$f_{c \perp} = = 258.0 \text{ psi}$$

$$f_{c\perp} = 258.0 \text{ psi} < F_{c\perp x'} = 650.00 \text{ psi} \text{ (CSI} = 0.40) ? OK$$

\*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The beam calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

Beam Design	Randy Ludwar	736 Mai	n Street, Box 614		Geno
N. Wilkerson	MEDEEK ENGINE			This report may not be copied, reproduced or distributed without the written consent of Medeek Engineering Inc.	Rev.
6/29/2022	3050 State Route 109 Copa ph. (425) 741-5555 www.r	lis Beach, WA 98535 nedeek.com		Copyright © 2014	Page 6