Beam Design

1. Beam Data

2.	Design	<u>1 Loads</u>
	<u> </u>	

Load Type:	Uniform Dist. Load	Live Lo
Support:	Simple Beam	Dead L
Beam Type:	Glulam	Selfwei
Species:	Western Species	Dist. Se
Grade:	24F-V4 1.8E DF/DF	Total W
Size:	5.5 x 15	
Design Span (L):	17.54 ft	•
Clear Span:	17.08 ft	•
Total Span:	18.00 ft	•
Bearing (lb):	5.5 ir	1.
Quantity (N):	1	

Live Load:	260	plf
Dead Load:	195	plf
Selfweight:	339.3	lbs
Dist. Selfweight:	19.34	plf
Total Weight:	348.2	lbs

3. Design Options

Lateral Support:	unbraced
Defl. Limits:	240 180
Load Duration:	1.25
Exposure:	dry
Temperature:	$T \leq 100^{\circ}F$
Orientation:	Vertical

4. Design Assumptions and Notes

Code Standard:	IBC 2015, NDS 2015
Bending Stress:	Parallel to Grain
Notes:	

5. Adjustment Factors

Factor	Description	Fb	Ft	Fv	Fc	$F_{c\perp}$	E/E _{min}
CD	Load Duration Factor	1.25	1.25	1.25	1.25	-	-
CM	Wet Service Factor	1	1	1	1	1	1
Ct	Temperature Factor	1	1	1	1	1	1
CL	Beam Stability Factor	0.946	-	-	-	-	-
Cv	Volume Factor	0.989 ^b	-	-	-	-	-
Cfu	Flat Use Factor	N/A ^c	-	-	-	-	-

a) Adjustment factors per AWC NDS 2015 and NDS 2015 Supplement.

b) The volume factor, CV, shall not apply simultaneously with the beam stability factor, CL. The lesser factor shall apply.

c) Only applies when glulam beam is loaded in bending about the y-y axis.

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6. Beam Calculations

Determine reference design values, sectional properties and self weight of beam:

A = b x d

,

,

where:

b = Breadth of rectangular beam in bending (in.)

d = Depth of rectangular beam in bending (in.)

A = Cross sectional area of beam $(in.^2)$

 S_x = Section modulus about the X-X axis (in.³)

 S_y = Section modulus about the Y-Y axis (in.³)

 I_x = Moment of inertia about the X-X axis (in.⁴)

 I_y = Moment of inertia about the Y-Y axis (in.⁴)

$$\begin{split} &b = 5.500 \text{ in.} \\ &d = 15.000 \text{ in.} \\ &A = 5.500 \text{ x } 15.000 = 82.50 \text{ in.}^2 \\ &S_x = (5.500)(15.000)^2/6 = 206.25 \text{ in.}^3 \\ &S_y = (5.500)^2(15.000)/6 = 75.63 \text{ in.}^3 \\ &I_x = (5.500)(15.000)^3/12 = 1546.88 \text{ in.}^4 \\ &I_y = (5.500)^3(15.000)/12 = 207.97 \text{ in.}^4 \end{split}$$

Reference Design Values from Table 5A NDS Supplement (Reference Design Values for Structural Glue Laminated Softwood Timber Combinations).

Species & Grade	Fbx+	Fbx-	$F_{c\perpx}$	Fvx	Ex	Eminx	Fby	$F_{c\perpy}$	Fvy	Ey	Eminy	Ft	Fc	G
24F-V4 1.8E DF/DF	2400	1850	650	265	1800000	950000	1450	560	230	1600000	850000	1100	1650	0.5

The following formula shall be used to determine the density of wood (lbs/ft³. (NDS Supplement Sec. 3.1.3)

where:

 ρ_w = Density of wood (lbs/ft³) G = Specific gravity of wood (dimensionless) m.c. = Moisture content of wood (percentile)

G = 0.5

m.c. = 16 % (Max. moisture content at dry service conditions)

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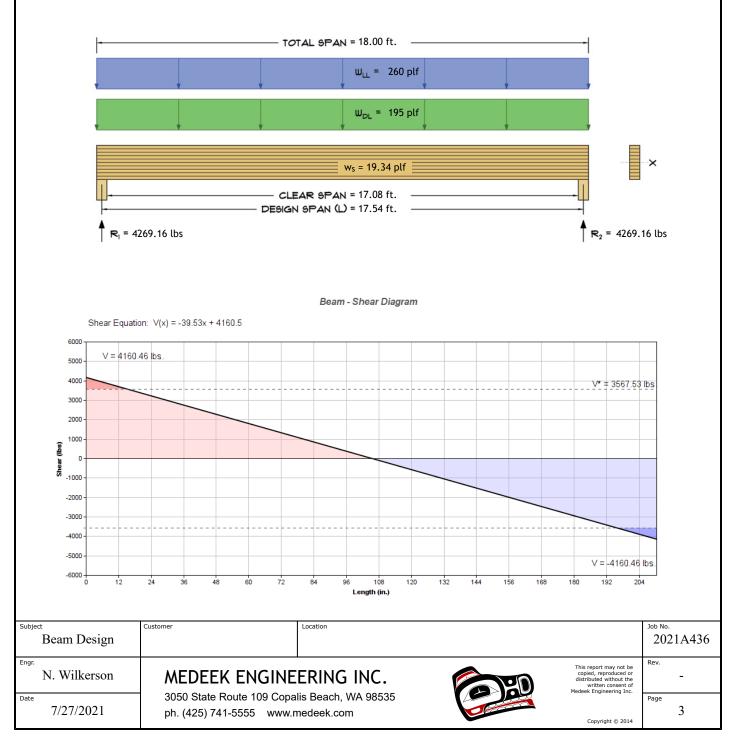
 $= 33.76 \text{ lbs/ft}^3$

Volume_{total} = N[A x (L + l_b)] = 1 x [82.50 x (210.50 + 5.5)] x (12 in./ft.)³ = 10.31 ft³ Volume_{span} = N[A x L] = 1 x [82.50 x 210.50] x (12 in./ft.)³ = 10.05 ft³

Total Weight (W_T) = $\rho_W x$ Volume_{total} = 33.76 x 10.31 = 348.2 lbs Self Weight (W_S) = $\rho_W x$ Volume_{span} = 33.76 x 10.05 = 339.3 lbs

Distributed Self Weight (w_s) = = 19.34 plf

Load, Shear and Moment Diagrams:





1.) Bending:

Members subject to bending stresses shall be proportioned so that the actual bending stress or moment shall not exceed the adjusted bending design value:

$f_b \leq F_b' \ (\textit{NDS Sec. 3.3.1})$

where:

$$\begin{split} \mathbf{f}_b &= \mathbf{M} \ / \ \mathbf{S} \\ F_{bx}' &= F_{bx}(\mathbf{C}_D)(\mathbf{C}_M)(\mathbf{C}_t)(\mathbf{C}_V) \quad \text{or} \quad F_{bx}' &= F_{bx}(\mathbf{C}_D)(\mathbf{C}_M)(\mathbf{C}_t)(\mathbf{C}_L) \end{split}$$

Beam is unbraced along its compression edge, lateral stability is considered below:

Slenderness Ratio for bending member RB:

 $l_u = Unbraced Length = 17.542$ ft.

 $l_u/d = = 14.03$

$$l_e = 1.63 l_u + 3d = 1.63(210.5) + 3(15) = 388.12$$
 in. = 32.34 ft. (NDS Table 3.3.3)

 $R_b = = 13.87$

 $R_b = 13.87 < 50 \ ? \ \textbf{OK}$

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Euler-based ASD critical buckling value for bending members:

$$E_{miny}' = E_{miny}(C_M)(C_t) = 850000(1)(1) = 850000 \text{ psi}$$

 $F_{bE} = = 5299.89 \text{ psi}$

 $F_{bx}^* = F_{bx}(C_D)(C_M)(C_t) = (2400)(1.25)(1)(1) = 3000.00 \text{ psi}$

Beam stability factor:

 $C_L = = 0.946$

 $C_{V} = = 0.989$

Lateral stability governs over volume effect:

$$C_L = 0.946 < C_V = 0.989$$

 $F_{bx'} = (2400)(1.25)(1)(1)(0.946) = 2836.7 \text{ psi}$

 $f_b = = 1061.6 \text{ psi}$

 $f_b = 1061.6 \text{ psi} < F_{bx'} = 2836.7 \text{ psi} (CSI = 0.37)$? **OK**

2.) Shear:

Members subject to shear stresses shall be proportioned so that the actual shear stress parallel to grain or shear force at any cross section of the bending member shall not exceed the adjusted shear design value:

 $f_v \leq F_v'$ (NDS Sec. 3.4.1)

where:

 $f_V =$

 $F_v' = F_v(C_D)(C_M)(C_t)$

 $F_{vx'} = (265)(1.25)(1)(1) = 331.25 \text{ psi}$

Shear Reduction: Uniformly distributed loads within a distance, d, from supports equal to the depth of the bending member shall be permitted to be ignored. Concentrated loads within a distance equal to the depth of the bending member from supports shall be permitted to be multiplied by x/d where x is the distance from the beam support face to the load. See NDS 2015, Figure 3C.

 $f_v * = = 64.86 \text{ psi}$

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 $f_v^* = 64.86 \text{ psi} < F_{vx'} = 331.25 \text{ psi} (CSI = 0.20)$? **OK**

No Reduction in Shear (conservative):

 $f_v = = 75.64 \text{ psi}$

 $f_v = 75.64 \text{ psi} < F_{vx'} = 331.25 \text{ psi} (CSI = 0.23)$? **OK**

3.) Deflection:

Bending deflections calculated per standard method of engineering mechanics for live load and total load:

LL Allowable: L/240 TL Allowable: L/180

 $E_x' = E_x(C_M)(C_t) = 1800000(1)(1) = 1800000 \text{ psi}$

 $\Delta_{LL}=$ = 0.20 in.

 $(L/d)_{LL} = 210.50 / 0.20 = 1058$

 $\Delta_{LL} = 0.20 \text{ in} = L/1058 < L/240$? **OK**

 ΔTL = $\,=$ 0.36 in.

 $(L/d)_{TL} = 210.50 / 0.36 = 580$

 $\Delta_{TL} = 0.36 \text{ in} = L/580 < L/180$? **OK**

4.) Bearing:

Members subject to bearing stresses perpendicular to the grain shall be proportioned so that the actual compressive stress perpendicular to grain shall be based on the net bearing area and shall not exceed the adjusted compression design value perpendicular to grain:

 $f_{c\perp} \leq F_{c\perp}$ ' (NDS Sec. 3.10.2)

where:

 $f_{c\,\perp} =$

 $F_{c\perp}' = F_{c\perp}(C_M)(C_t)$

 $F_{c \perp x}' = (650)(1)(1) = 650.00 \text{ psi}$

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 $A_b = b \ x \ l_b = 5.5 \ x \ 5.5 = 30.25 \ in^2$

 $f_{c\,\perp}=\,=141.1~psi$

 $f_{c\,\perp}$ = 141.1 $psi < F_{c\,\perp\,x'}$ = 650.00 psi~(CSI = 0.22) ? OK

*Disclaimer: The calculations produced herein are for initial design and estimating purposes only. The calculations and drawings presented do not constitute a fully engineered design. All of the potential load cases required to fully design an actual structure may not be provided by this calculator. For the design of an actual structure, a registered and licensed professional should be consulted as per IRC 2012 Sec. R802.10.2 and designed according to the minimum requirements of ASCE 7-10. The beam calculations provided by this online tool are for educational and illustrative purposes only. Medeek Design assumes no liability or loss for any designs presented and does not guarantee fitness for use.

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