Surface Roughness B: Urban and suburban areas, wooded areas, or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.

Surface Roughness C: Open terrain with scattered obstructions having heights generally less than 30 ft ( 9.1 m ). This category includes flat open country and grasslands.

Surface Roughness D: Flat, unobstructed areas and water surfaces. This category includes smooth mud flats, salt flats, and unbroken ice.

### 26.7.3 Exposure Categories

Exposure B: For buildings with a mean roof height of less than or equal to $30 \mathrm{ft}(9.1 \mathrm{~m})$, Exposure B shall apply where the ground surface roughness, as defined by Surface Roughness B, prevails in the upwind direction for a distance greater than 1,500 ft ( 457 m ). For buildings with a mean roof height greater than $30 \mathrm{ft}(9.1 \mathrm{~m})$, Exposure B shall apply where Surface Roughness B prevails in the upwind direction for a distance greater than $2,600 \mathrm{ft}(792 \mathrm{~m})$ or 20 times the height of the building, whichever is greater.

Exposure C: Exposure C shall apply for all cases where Exposures B or D do not apply.

Exposure D: Exposure D shall apply where the ground surface roughness, as defined by Surface Roughness D, prevails in the upwind direction for a distance greater than $5,000 \mathrm{ft}(1,524 \mathrm{~m})$ or 20 times the building height, whichever is greater. Exposure D shall also apply where the ground surface roughness immediately upwind of the site is B or C , and the site is within a distance of $600 \mathrm{ft}(183 \mathrm{~m})$ or 20 times the building height, whichever is greater, from an Exposure D condition as defined in the previous sentence.

For a site located in the transition zone between exposure categories, the category resulting in the largest wind forces shall be used.

EXCEPTION: An intermediate exposure between the preceding categories is permitted in a transition zone provided that it is determined by a rational analysis method defined in the recognized literature.

### 26.7.4 Exposure Requirements.

### 26.7.4.1 Directional Procedure (Chapter 27)

For each wind direction considered, wind loads for the design of the MWFRS of enclosed and partially enclosed buildings using the Directional Procedure of Chapter 27 shall be based on the exposures as defined in Section 26.7.3. Wind loads for the design of open buildings with monoslope, pitched, or troughed free roofs shall be based on the expo-
sures, as defined in Section 26.7.3, resulting in the highest wind loads for any wind direction at the site.

### 26.7.4.2 Envelope Procedure (Chapter 28)

Wind loads for the design of the MWFRS for all low-rise buildings designed using the Envelope Procedure of Chapter 28 shall be based on the exposure category resulting in the highest wind loads for any wind direction at the site.

### 26.7.4.3 Directional Procedure for Building Appurtenances and Other Structures (Chapter 29)

Wind loads for the design of building appurtenances (such as rooftop structures and equipment) and other structures (such as solid freestanding walls and freestanding signs, chimneys, tanks, open signs, lattice frameworks, and trussed towers) as specified in Chapter 29 shall be based on the appropriate exposure for each wind direction considered.

### 26.7.4.4 Components and Cladding (Chapter 30)

Design wind pressures for components and cladding shall be based on the exposure category resulting in the highest wind loads for any wind direction at the site.

### 26.8 TOPOGRAPHIC EFFECTS

### 26.8.1 Wind Speed-Up over Hills, Ridges, and Escarpments

Wind speed-up effects at isolated hills, ridges, and escarpments constituting abrupt changes in the general topography, located in any exposure category, shall be included in the design when buildings and other site conditions and locations of structures meet all of the following conditions:

1. The hill, ridge, or escarpment is isolated and unobstructed upwind by other similar topographic features of comparable height for 100 times the height of the topographic feature $(100 \mathrm{H})$ or 2 mi ( 3.22 km ), whichever is less. This distance shall be measured horizontally from the point at which the height $H$ of the hill, ridge, or escarpment is determined.
2. The hill, ridge, or escarpment protrudes above the height of upwind terrain features within a 2-mi $(3.22-\mathrm{km})$ radius in any quadrant by a factor of two or more.
3. The structure is located as shown in Fig. 26.8-1 in the upper one-half of a hill or ridge or near the crest of an escarpment.


Topographic Factor, $\mathbf{K}_{\mathbf{z t}}$
Figure 26.8-1 (cont'd)

## Equations:

$$
\mathrm{K}_{\mathrm{zt}}=\left(1+\mathrm{K}_{1} \mathrm{~K}_{2} \mathrm{~K}_{3}\right)^{2}
$$

$\mathrm{K}_{1}$ determined from table below
$K_{2}=\left(1-\frac{|\mathrm{x}|}{\mu \mathrm{L}_{\mathrm{h}}}\right)$
$K_{3}=e^{-\gamma z / L_{h}}$

| Parameters for Speed-Up Over Hills and Escarpments |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hill Shape | $\mathrm{K}_{1} /\left(\mathbf{H} / \mathrm{L}_{\mathrm{h}}\right)$ |  |  | $\gamma$ | $\mu$ |  |
|  | Exposure |  |  |  | Upwind of Crest | Downwind of Crest |
|  | B | C | D |  |  |  |
| 2-dimensional ridges (or valleys with negative H in $\mathrm{K}_{1} /\left(\mathrm{H} / \mathrm{L}_{\mathrm{h}}\right)$ | 1.30 | 1.45 | 1.55 | 3 | 1.5 | 1.5 |
| 2-dimensional escarpments | 0.75 | 0.85 | 0.95 | 2.5 | 1.5 | 4 |
| 3-dimensional axisym. hill | 0.95 | 1.05 | 1.15 | 4 | 1.5 | 1.5 |

4. $H / L_{h} \geq 0.2$.
5. $H$ is greater than or equal to $15 \mathrm{ft}(4.5 \mathrm{~m})$ for Exposure C and D and $60 \mathrm{ft}(18 \mathrm{~m})$ for Exposure B.

### 26.8.2 Topographic Factor

The wind speed-up effect shall be included in the calculation of design wind loads by using the factor $K_{z t}$ :

$$
\begin{equation*}
K_{z t}=\left(1+K_{1} K_{2} K_{3}\right)^{2} \tag{26.8-1}
\end{equation*}
$$

where $K_{1}, K_{2}$, and $K_{3}$ are given in Fig. 26.8-1.
If site conditions and locations of structures do not meet all the conditions specified in Section 26.8.1 then $K_{z t}=1.0$.

### 26.9 GUST-EFFECTS

26.9.1 Gust-Effect Factor: The gust-effect factor for a rigid building or other structure is permitted to be taken as 0.85 .

### 26.9.2 Frequency Determination

To determine whether a building or structure is rigid or flexible as defined in Section 26.2, the fundamental natural frequency, $n_{1}$, shall be established using the structural properties and deformational characteristics of the resisting elements in a properly substantiated analysis. Low-Rise Buildings, as defined in 26.2, are permitted to be considered rigid.

### 26.9.2.1 Limitations for Approximate Natural Frequency

As an alternative to performing an analysis to determine $n_{1}$, the approximate building natural frequency, $n_{a}$, shall be permitted to be calculated in accordance with Section 26.9.3 for structural steel, concrete, or masonry buildings meeting the following requirements:

1. The building height is less than or equal to 300 ft (91 m), and
2. The building height is less than 4 times its effective length, $L_{\text {eff. }}$.

The effective length, $L_{\text {eff }}$, in the direction under consideration shall be determined from the following equation:

$$
\begin{equation*}
L_{e f f}=\frac{\sum_{i=1}^{n} h_{i} L_{i}}{\sum_{i=1}^{n} h_{i}} \tag{26.9-1}
\end{equation*}
$$

The summations are over the height of the building where
$h_{i}$ is the height above grade of level $i$
$L_{i}$ is the building length at level $i$ parallel to the wind direction

### 26.9.3 Approximate Natural Frequency

The approximate lower-bound natural frequency $\left(n_{a}\right)$, in Hertz, of concrete or structural steel buildings meeting the conditions of Section 26.9.2.1, is permitted to be determined from one of the following equations:

For structural steel moment-resisting-frame buildings:

$$
\begin{equation*}
n_{a}=22.2 / h^{0.8} \tag{26.9-2}
\end{equation*}
$$

For concrete moment-resisting frame buildings:

$$
\begin{equation*}
n_{a}=43.5 / h^{0.9} \tag{26.9-3}
\end{equation*}
$$

For structural steel and concrete buildings with other lateral-force-resisting systems:

$$
\begin{equation*}
n_{a}=75 / h \tag{26.9-4}
\end{equation*}
$$

For concrete or masonry shear wall buildings, it is also permitted to use

$$
\begin{equation*}
n_{a}=385\left(\mathrm{C}_{w}\right)^{0.5} / h \tag{26.9-5}
\end{equation*}
$$

where

$$
C_{w}=\frac{100}{A_{B}} \sum_{i=1}^{n}\left(\frac{h}{h_{i}}\right)^{2} \frac{A_{i}}{\left[1+0.83\left(\frac{h_{i}}{D_{i}}\right)^{2}\right]}
$$

where
$h=$ mean roof height (ft)
$n=$ number of shear walls in the building effective in resisting lateral forces in the direction under consideration
$A_{B}=$ base area of the structure $\left(\mathrm{ft}^{2}\right)$
$A_{i}=$ horizontal cross-section area of shear wall " $i$ " $\left(\mathrm{ft}^{2}\right)$
$D_{i}=$ length of shear wall " $i$ " (ft)
$h_{i}=$ height of shear wall " $i$ " (ft)

### 26.9.4 Rigid Buildings or Other Structures

For rigid buildings or other structures as defined in Section 26.2, the gust-effect factor shall be taken as 0.85 or calculated by the formula:

$$
\begin{gather*}
G=0.925\left(\frac{1+1.7 g_{Q} I_{\bar{z}} Q}{1+1.7 g_{v} I_{\bar{z}}}\right)  \tag{26.9-6}\\
I_{\bar{z}}=c\left(\frac{33}{\bar{z}}\right)^{1 / 6} \tag{26.9-7}
\end{gather*}
$$

