

A REVIEW OF CLIMATOLOGICAL DATA FOR  
GROUND SNOW LOADS IN ARIZONA

by

Scott Brent Freestone

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APPROVAL BY REPORT DIRECTOR

This report has been approved on the date shown below:

\_\_\_\_\_  
Achintya Haldar, Ph.D., P.E.  
Professor of Civil Engineering  
And Engineering Mechanics

\_\_\_\_\_  
Date

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*ABSTRACT*A REVIEW OF CLIMATOLOGICAL DATA  
FOR GROUND SNOW LOADS IN ARIZONA

by Scott Brent Freestone

Snow loads on structures are determined in modern building codes by applying ground-to-roof conversion coefficients to ground snow loads. An essential part of structural design for the mountainous regions of Arizona is designing for loads due to snow. For accurate design for snow loading current ground snow loads in Arizona must be updated. Historical snow data obtained from climate monitoring stations throughout the state provide a basis for ground snow load predictions. Historical snow depth and snow water equivalent data from climate monitoring sources for over 500 sites across the state of Arizona have been compiled and modeled with lognormal distributions to determine ground snow loads to be used in structural design. Ground snow loads with a mean recurrence interval of fifty and thirty years (2% and 3.3% annual probability of exceedance, respectively) are presented.

## ***CHAPTER ONE***

### **INTRODUCTION**

#### **1.1 CODE BACKGROUND**

When approaching the design of a new structure, one generally refers to a building code, such as the *International Building Code* (IBC) which is very common in urbanized areas in Arizona, or the *Uniform Building Code* (UBC) which is still enforced in some rural jurisdictions in Arizona. Building codes give an outline of the design process for the structure, from the applied loads to account for to the analysis procedure to employ. Second only to the utility required by the users of the structure, the loads imposed on the structure are often the starting point of the design. Individual loads will affect a structure independently and simultaneously in combinations. In most current building codes combinations of loads to consider are given for two methods of design, *Allowable Stress Design* (ASD, also referred to as *Allowable Strength Design* or *Working Stress Design*) and *Load and Resistance Factor Design* (LRFD). (ASCE 7-05)

ASD is characterized by comparing the stresses on structural members due to applied design loads to a predetermined allowable stress for the structural members. These allowable stresses are established based on safety factors applied to the material strength exhibited in standardized testing procedures and historical performance. These factors are applied to the material strength to provide a confidence against the uncertainty inherent in the material strength. These factors vary depending on the failure mechanism (e.g., bending moment, shear, axial compression, etc.)

The common load combinations used for structural design with ASD are:

$$Dead \quad (1.1)$$

$$Dead + Live \quad (1.2)$$

$$Dead + Live + (Roof Live or Snow or Rain) \quad (1.3)$$

$$Dead + (Wind or 0.7Seismic) + Live + \\ (Roof Live or Snow or Rain) \quad (1.4)$$

$$0.6Dead + Wind \quad (1.5)$$

$$0.6Dead + 0.7Seismic \quad (1.6)$$

LRFD is characterized by comparing the strength requirements of the structural components due to the application of factored loads to the allowable strength of the member. Each type of load (e.g., dead, roof live, roof snow, etc.) has a different factor representing uncertainty in the magnitude of the load and in the analysis from which the load effect is obtained from the load. (Steel, 2005) As in ASD, the strength of the member also has different resistance factors applied corresponding to each failure mechanism.

The common load combinations used in structural design with LRFD are:

$$1.4Dead \quad (1.7)$$

$$1.2Dead + 1.6Live + 0.5(Roof Live or Snow or Rain) \quad (1.8)$$

$$1.2Dead + 1.6(Roof Live or Snow or Rain) + \\ (f_1Live or 0.8Wind) \quad (1.9)$$

$$1.2Dead + 1.6Wind + f_1Live + \\ 0.5(Roof Live or Snow or Rain) \quad (1.10)$$

$$1.2Dead \pm 1.0Seismic + f_1Live + f_2Snow \quad (1.11)$$

$$0.9Dead \pm (1.6Wind or 1.0Seismic) \quad (1.12)$$

where  $f_1 = 1.0$  for floors in places of public assembly, for live loads in excess of 100 psf, and for parking garage live loads.

$= 0.5$  for other live loads.

$f_2 = 0.7$  for roof configurations (such as saw tooth) that do not



shed snow off the structure.  
 = 0.2 for other roof configurations.

Along with the affects of Dead, Live, Wind and Seismic loading, the affects of Snow on a structure can be a crucial aspect of the design. In both design approaches, ASD and LRFD, the design snow load is calculated from a basic ground snow load. The equation for determining the snow load to be used in the design of a building with a roof of a very low pitch (referred to as a flat roof in building codes) is as follows:

$$P_f = 0.7C_e C_t I p_g \quad (1.13)$$

where  $P_f$  is the Flat Roof Snow Load (psf)  
 $C_e$  is a dimensionless exposure factor  
 $C_t$  is a dimensionless thermal factor  
 $I$  is a dimensionless importance factor  
 $p_g$  is the ground snow load (psf)

The determination of the roof snow load for sloped roofs has an additional step:

$$P_s = C_s P_f \quad (1.14)$$

where  $C_s$  is a Roof Slope Factor  
 $P_f$  is the Flat Roof Snow Load determined in equation 1.13

The Roof Slope Factor takes into account whether the roof is warm or cold, whether the surface of the roof is slippery and unobstructed or not, how much pitch the roof has and whether the roof is curved or not. (ASCE 7-05)

## 1.2 MOTIVATION

When considering structural design and engineering in Arizona, where the warm weather and dry climate are well known, the last thing typically thought of is the affects of snow on a structure. Much of the populated portions of the state rest at elevations lower than 4,500 feet (1,372 m) above mean sea level and rarely have a considerable accumulation of snow.



Figure 1.1  
Looking West down the University of Arizona Mall  
In Tucson during a Snow Storm on April 4, 1999

Even in most of the areas where it does snow, the load from the small amount of snow that is likely to accumulate would not exceed the live load consideration required in the load combinations previously mentioned. From these load combinations it can be surmised that if the roof snow load is equal to or less than the roof live load, there is no need to consider the snow load at all. (It is important to note that for large roof areas the live load may be reduced, but roof snow load may not. In these cases, the roof snow load

may be negligible if it is less than the reduced roof live load, 12 psf in most cases.) (IBC, 2003) This is the case for most of the metropolitan areas in the state. There are however locations in the mountainous regions of the state, where this is not the case. Snow depths have been reported as high as 83 inches for Flagstaff, Arizona. (NCDC, 2006) This is a city with a population of about 57,000. Among other things, Flagstaff is the home of Northern Arizona University with approximately 19,000 students.



Figure 1.2  
Apartments in Flagstaff after a Snow Storm

There are also rural areas in Arizona affected by snow loading. Although these areas are not the most urbanized regions of the state, there are some very significant structures in these areas that could fail due to the load of snow accumulation if it was not

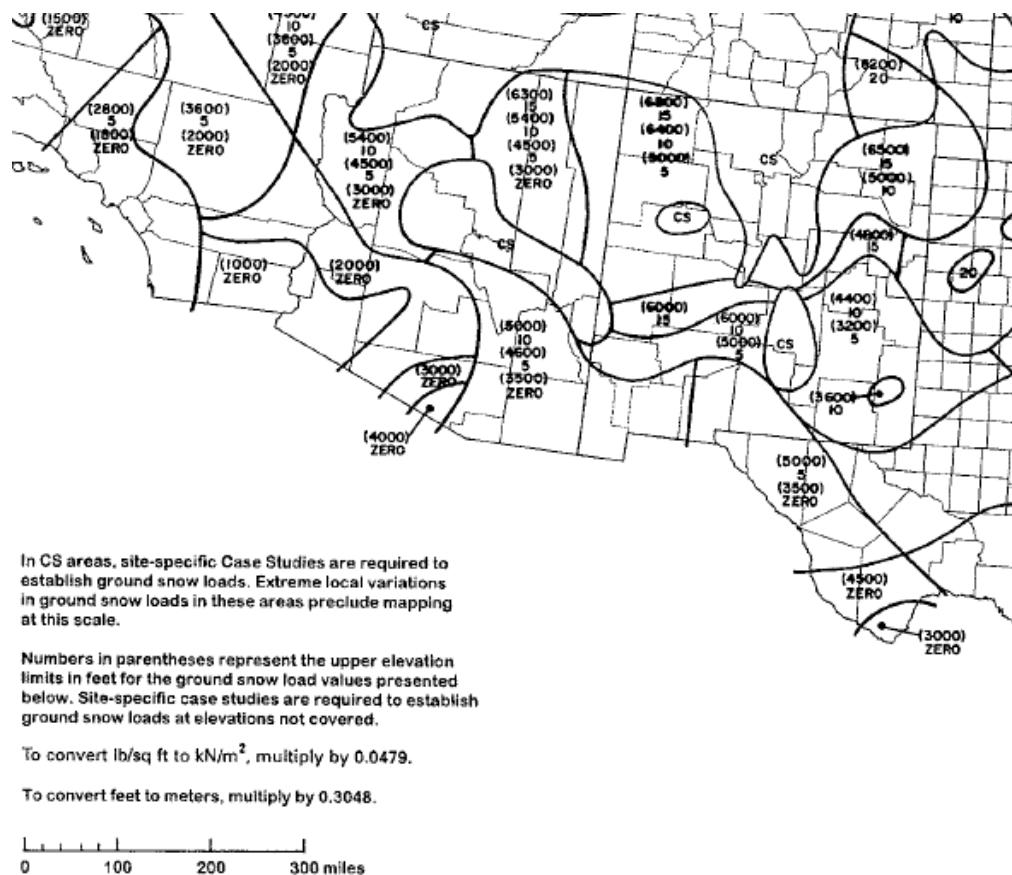
properly considered in the design. For instance, in Southern Arizona, an area where the climate is generally warm and snow loading is not given much thought in structural design, Mt. Graham is the home of the University of Arizona's Large Binocular Telescope (LBT) at an elevation of 10,480 feet (3,194 m) above mean sea level. The enclosure for the LBT is the height of a 12-story building rotating on a cylindrical base 38 feet tall. This is a very significant structure that houses "the world's most powerful optical telescope" (World's, 2003) where reported snow depths have been as much as 120 inches (Elliott, 1981).

Another rural location with significant construction is a telescope site in Happy Jack, Arizona. Happy Jack is a small town with a population between 600 and 700 in the southeast corner of Coconino County. Lowell Observatory in conjunction with The Discovery Channel is currently in the process of building a 4.2-meter telescope at an altitude of 7800 feet (2,377 m) above mean sea level in Happy Jack. Because Happy Jack is a small rural town there is no design snow load determined by a local jurisdiction for structures built in the area. In addition to these telescope sites, there are also mines with large structures at rural sites throughout Arizona, some of which are at high elevations where snow would contribute significantly to the loading on the structure.

Loading due to snow is the most critical design load for many structures in the mountainous regions of Arizona. At higher elevations snow will accumulate for most of the winter season, and although Arizona boasts of warmer and dryer climates than much of the United States, snow depths have been recorded as high as 126 inches (Elliott,

1981). The presence of significant amounts of snow during the winter months requires the use of accurate snow loads in the design of buildings and other structures.

Some guidance for snow load design in Arizona does currently exist. Most building codes currently used, such as the IBC and UBC reference snow load maps published by the American Society of Civil Engineers (ASCE) in the Standard for Minimum Design Loads for Buildings and Other Structures, ASCE 7. The maps in ASCE 7 indicate basic ground snow loads to be used in design,  $p_g$  used to determine the roof snow load in equation 1.13. These maps have contours separating regions of different climatological behavior. In each region grouped by contour lines the basic ground snow load is indicated based on the elevation. At elevations above those shown or locations where the snow load is unknown the designation “CS” is given to indicate that a “Case Study” of that specific area is required to determine the basic ground snow load to be used for design. Because of the characteristics of the rapidly varying terrain in the mountainous regions of Arizona, the snow load cannot always be accurately shown by general smoothed isolines. Hence, the “Case Study” areas are extensive for the higher elevation areas of the state. This makes it difficult to design for snow loads in these areas without performing an in-depth case study to determine the snow loads. (Tobiasson and Greatorrex, 1996)



**FIGURE 7-1**  
**GROUND SNOW LOADS,  $p_g$  FOR THE UNITED STATES (LB/SQ FT)**

Figure 1.3  
 A Portion of the ASCE 7-05 Ground Snow Load Map

To provide more accurate local information regarding ground snow loads many states have published their own snow load design guides. (Arizona, 1981, Oregon, 1971, Washington, 1995) Arizona's state-specific guide, Snow Load Data for Arizona, provides another resource for determining ground snow loads. (Elliot, 1981) In this study ground snow loads with a magnitude predicted to occur once every 30 years are published as a guide to aid in structural design.

Although we have a few resources available to assist in determining snow loads for structural design, there still exists a need for additional information as well as further refinement of existing information. Much of the map in ASCE 7 indicates requirements for case studies to determine the ground snow load for specific areas. Snow Load Data for Arizona gives recommendations for snow loads based on a 30-year return period, which is not consistent with the requirements for most modern building codes. (ASCE 7-05) Furthermore, there has been much research in this area over the past three decades that could improve current snow load design methods. In Snow Load Data for Arizona it is suggested that a future publication would be advisable as more weather information and research becomes available (Elliott, 1981). This report explores the research in snow loading from the 1980's to current and its application in structural design in Arizona.

### 1.3 OBJECTIVES

There is a recognized need for guidance when considering the effects of snow loads on structures in the higher elevations of Arizona. Even though some locations in Arizona have experienced considerable amounts of snowfall, the current resources available do not provide a complete guide to designing structures in Arizona for snow loads. In this study, reliable snow loads are developed for use in the design of buildings and other structures in Arizona from historical climatological data. The National Water and Climate Center (NWCC), under the U.S. Department of Agriculture National Resources Conservation Service (formerly the U.S. Soils Conservation Service), has kept

records for the amount of snow on the ground measured in *Snow Water Equivalents* (SWE) for 56 measuring stations in the mountainous regions of Arizona. The National Climate Data Center (NCDC) also has an extensive database of historical weather records that includes measurements for the daily snow depth at over 480 sites in Arizona.

Historical records for each of the sites with SWE data are statistically analyzed to determine an annual extreme SWE that has a mean recurrence interval of 50 years (or an annual 2% probability of being exceeded). (Halдар and Mahadevan, 2000) The SWE is then translated directly into a snow load to give a ground snow load that will occur on average once every 50 years, or a 50-year basic ground snow load. Likewise, the historical records for daily snow depth are analyzed to predict an annual extreme snow depth that has a mean recurrence interval of 50 years. An equation relating snow density to the depth is then applied to the 50-year ground snow depth to determine a 50-year ground snow load. Predicting 50-year ground snow loads in this report is consistent with current codes and building design practices. (ASCE 7-05)

The Happy Jack site mentioned previously is in a designated Case Study area in the ASCE 7-05 ground snow load maps and has a recommended ground snow load of 95 psf in Snow Load Data for Arizona. (Elliott, 1981) It will be interesting to compare this ground snow load value to that found with the updated modeling to see if the previous ground snow load is consistent with current practices suggested in the code.



## ***CHAPTER TWO***

### **LITERATURE REVIEW**

#### **2.1 OVERVIEW**

Snow Load Data for Arizona (Elliott, 1981) provides useful snow load information for many specific sites in Arizona and is widely used today as a guide by design consultants across the state. The methods used in the development of ground snow loads have received considerable attention in the research community and have improved substantially during the past 3 decades. Since the publication of Snow Load Data for Arizona there has been much research done on the statistical modeling of snow loads. There have also been developments in the determination of ground snow loads from historical data with only depth of snow records. Furthermore, there now exists approximately 32 additional years of historical snow records with improvements on measuring and reporting the data.

#### **2.2 SNOW LOAD DATA FOR ARIZONA**

Snow Load Data for Arizona (Elliott, 1981) was prepared by a special Snow Load Committee of the Central Chapter of the Structural Engineers Association of Arizona in Cooperation with the Civil Engineering Department of Arizona State University. It was published in 1973 with a second printing in 1981. There were no revisions to the information in the second printing, only a one-page Foreword with a few notes responding to frequent questions that had been voiced as the structural design community of Arizona had begun implementing the findings of the report in their work.

Snow Load Data for Arizona presents basic ground snow loads to be used in design as well as information for the application of snow loads on structures with factors for wind exposure, sloped roofs, shape of roofs, etc. similar to those outlined in Section 1.1. The ground snow loads presented will be discussed here, but the application of loads on structures will be ignored since this is covered thoroughly in building codes and is beyond the scope of this study.

The introduction to Snow Load Data for Arizona states that it is intended to serve as a guide to structural designers. As a guide to designers there is no statistical information detailed in the report. There is however tables giving useful information including the 30-year basic ground snow load and the 30-year basic roof snow load for each of the sites studied. These tables also list the elevation for each site, the month and year of the maximum snow depth; maximum snow depth (inches), maximum measured, calculated, or estimated weight of snow on ground (psf) and the source of the data. Each site in the tables is divided into one of five “Snow Zones.” The Snow Zones are geographical areas of Arizona that are separated by weather patterns and weather behavior, including snowfall.

The report states that the three major sources of snow data used were: 1) U.S. Soil Conservation Service (SCS) Snow Survey records, 2) U.S. Weather Bureau records and 3) “Actual Snow Loads in Arizona, a detailed study of the great storm of December 1967.” (Elliott, 1968) The SCS records provided data for the depth and weight of snow on the ground. Actual Snow Loads in Arizona provided information for snow depth and estimated ground snow loads for a single storm in December 1967. “This storm was the

heaviest short period snow fall in most areas of the state.” (Elliott, 1968) The U.S. Weather Bureau records provided information only for the depth of snow. For the data with only snow depth, weights of snow were determined by comparing similar stations that had both depth and weight measurements.

Plots for maximum ground snow loads vs. elevation for each Snow Zone are included as figures in the report. In discussion of the development of the snow loads it is stated that the original intent was to draw regression curves for each of these plots and use the curve values for the basic ground snow loads. This approach was abandoned due to inaccurate loads resulting from the wide scatter of data. No regression curves were included in the final publication of the report, only the plots. These curves did however aid in determining the ground snow loads, but ultimately each site was considered and assigned a ground snow load individually. Elliott does not indicate the precise methods used to determine the ground snow loads published in this report. He does indicate that the snow density assumptions for the sites with only snow depth measurements are most likely “fraught with error...There is no way of judging accuracies of these weight estimates, but hopefully they are within 30%±.” (Elliott, 1981)

Factors affecting the accuracy of determined snow loads based on depths are the variability in exposure of each of the sites (sun exposure decreases snow depth, but increases snow density) and the time during the season when the snow depth is measured (higher densities as snow melts later in the spring). Elliott suggests that reason and judgment were essential to develop the snow loads. If reason and judgment were required to determine snow loads based on the limited and varying data available, Elliott

would be considered a qualified judge since his study was the source of much of the data used. He suggests that reason and judgment were not only used to develop the loads, but they are also required in application of the snow loads in structural design.

### 2.3 RELIABLE STATISTICAL MODELS FOR SNOW

For sufficient design of structures it is imperative to have accurate snow loads for design. Snowfall, however, is a naturally occurring phenomenon with some variability. As is well known in Arizona where water resources depend considerably on snowmelt, snowfall or snow depth does not measure the same each season. That is to say, the snowfall or snow depth varies from season to season, and from site to site. In order to accurately predict snow loading for structural design there must be a reliable statistical model for this natural phenomenon.

The American National Standard A58 (the predecessor to the current design load standard, ASCE 7) developed ground snow loads for structural design based on an analysis of annual extreme ground snow load data from National Weather Service climate monitoring stations. The ground snow load maps in the 1972 publication of the A58 Standard were based on 10 years of data at 140 monitoring stations (Ellingwood and Redfield, 1983). Subsequent publications have increased the data used in analysis, in both number of years of data and number of sites. With this expansion of data the design values changed, in some cases these changes were significant. In the case of the 1972 Edition of the A58 Standard ground snow loads with a mean recurrence interval of 50 years were determined using only 10 years of data. It is easy to see that the extrapolation

from 10 years of actual data to predicting a value with a 50-year mean recurrence interval would be very sensitive to the statistical model used. The limited historical climatological data available from some of the climate monitoring stations makes it critical to find a reliable statistical model to predict the values to be used in analysis and design.

Some of the statistical distributions used for modeling of annual extreme snow loads in the era of the 1972 Edition of the A58 Standard were Tippet Type I and Frechét Type II extreme value distributions. These distributions were found to be inadequate in predicting the upper quantiles; the Type I distribution underestimated snow loads while the Type II distribution greatly overestimated the snow loads in the upper quantiles. Instead, the lognormal distribution was tried on the annual extreme snow water equivalent data for 50 sites across the United States and “fit very well” (Thom, 1966). The recommendation by Thom to use a lognormal distribution became the basis for the snow load maps in the 1972 Edition of the A58 Standard (Ellingwood and Redfield, 1984).

Ellingwood and Redfield (1983) further explored possible distributions used to model ground snow loads. Records for 76 “first-order” weather stations operated by the National Oceanic and Atmospheric Administration located in northeastern United States were examined. These sites are indicated as “first-order” stations because they provide data for the depth of snow as well as the water equivalent of snow (from which the weight of snow can be directly determined). The snow water equivalent (SWE) data was used to avoid the additional source of error inherent in the variability of the density

conversion of snow depth to snow load. In order to determine an appropriate distribution model the comparison was made between models that were commonly used to describe the distribution of the annual extreme SWE. The distributions included in the comparison in the study were lognormal, log-Pearson Type III, Type I and Type II. Because the common tests for goodness-of-fit, the chi-square for lognormal distribution and Kolmogorov-Smirnov for Type I distributions (Haldar and Mahadevan, 2000), gave conflicting results for the best fitting distributions, the maximum probability plot correlation coefficient (MPPCC) criterion was used to determine the goodness of fit for each distribution (Ellingwood and Redfield, 1984).

The results of the study indicated that according to the MPPCC criterion, 30% of the sites had a distribution best fit by a lognormal distribution, 21% by a log-Pearson Type III, 24% by a Type I and 25% by a Type II distribution. Although the number of sites which showed the best fit for each distribution assumption did not vary greatly, through further investigation data sets were developed from a lognormal parent population using Monte Carlo techniques (Haldar and Mahadevan, 2000) and found strikingly similar results for percentages of data sets best fit by the respective distributions. The exercise was repeated with a Type I parent population and altogether different results were found. Finally, the study was concluded in agreeance with the aforementioned report completed 18 years previous (Thom, 1966) with a recommendation to use lognormal distribution for modeling annual extreme snow water equivalents.

## 2.4 DENSITIES FOR DEPTH-ONLY SNOW DATA

The density of snow is a random variable that must be given consideration in determining ground snow loads from historical snow depth data. Tobiasson and Greatorex (1996) developed a method for relating the density of snow to its depth. To develop this relationship data was taken from historical observations for 266 “first order” National Weather Services stations across the United States that had records for both snow depth and SWE. All sites for which less than ten years of data were recorded or sites that had more than ten years of recorded data but less than five in which snow was observed were discarded. For each of the 204 sites remaining that fit these criteria, the maximum snow depth was determined for each winter and the maximum SWE was determined for each winter. Consistent with the recommendations discussed in Chapter 2.3, the distributions for annual maximum snow depth and for annual maximum SWE were assumed to be lognormal. (Thom, 1966, Ellingwood and Redfield, 1984) Based on a lognormal distribution a depth of snow on the ground with a 2% annual probability of being exceeded (or 50-year mean recurrence interval) was determined for each site. Likewise, a ground snow load with a 2% annual probability of being exceeded was determined for each site. The 50-year snow depths were plotted against the 50-year ground snow loads and the best-fit equation was nonlinear:

$$p_g = 0.279h_g^{1.36} \quad (2.1)$$

where:  $p_g$  is the ground snow load in psf  
 $h_g$  is the depth of snow in inches. (Tobiasson and Greatorex, 1996)

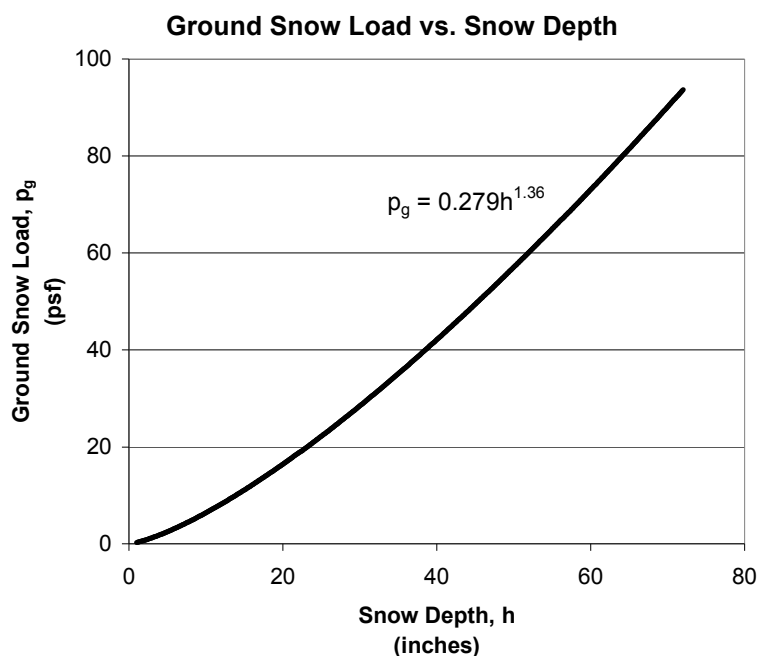


Figure 2.1  
Ground Snow Load Relationship to Snow Depth

Figure 2.1 illustrates that the density of snow increases with its depth. For instance, a snow depth of 20 inches contains 15.8% water while a snow depth of 60 inches contains 23.3% water. This seems to be an accurate assumption for sites where the snow continues to pack throughout the winter and spring, compressing snow near the bottom of the snow pack. As will be shown later, this may not be the most accurate relationship for sites that have an annual extreme snow depth characterized by a periodic snow storm that melts away before the next snow storm occurs.

This equation relating snow depth to ground snow load has been used for the forty-eight contiguous states to determine the ground snow loads from historical snow depth data to develop the ground snow load maps in the ASCE 7. (O'Rourke, 2004)



### *CHAPTER THREE*

#### DETERMINATION OF GROUND SNOW LOADS

##### 3.1 INTRODUCTION

The methods used to determine the ground snow loads presented in this report include gathering data for historical snow depth or snow water equivalents for as many sites as possible within the state of Arizona, modeling them statistically and determining a 50-year snow depth or 50-year snow water equivalent. For the snow depth data, density conversion is required to obtain a ground snow load. For the snow water equivalent data, a unit conversion from inches of water to pounds per square foot is required to determine the applicable load:

$$p_g = SWE \left( \frac{62.4 \frac{lbs}{cu.ft.}}{12 \frac{in.}{ft.}} \right) \quad (3.1)$$

where  $p_g$  is the ground snow load (psf)  
 $SWE$  is the Snow Water Equivalent (inches of water)

In order to reduce the task of analysis, it was decided to ignore data from certain sites that would obviously not have a ground snow load large enough to impact structural design. To be sure that the data would not yield a significant ground snow load a low threshold was set for those data sets to be ignored. All sites with an annual maximum observed snow depth of 6 inches or less were set aside and not included in the statistical

analysis. These sites are shown with a ground snow load equal to zero in Table 3.4, Summary of Ground Snow Loads, presented later in this chapter.

### 3.2 DATA COLLECTION

Data was taken from 2 main sources: the National Climate Data Center and the National Weather and Climate Center. These sources were used because they provided data that was publicly available and provided a large number of sites across the state. Other sources likely exist with additional historical snow depth and/or snow water equivalent data. The United States Forest Service was contacted for further historical snow data from the national forests but no response was received within time to include in this report. There are sites in Snow Data for Arizona for which current data could not be obtained. Although there is possibly more data available, the sources aforementioned seem to provide a good representation of Arizona and were utilized in this study.

Each site is assigned to a Climate Division, as illustrated in Figure 3.1, by the NCDC. Climate Divisions are intended to be geographic divisions of homogeneous weather. The Climate Divisions are a further development of the “Snow Zones” mentioned in Snow Load Data for Arizona. (Elliot, 1981) It appears that the boundaries for the climate divisions fall on county borders, which are not necessarily precise divisions for change in climate behavior. This method was likely chosen for convenient bookkeeping and reporting instead of homogenous climate. It should be noted that the NCDC is currently in the midst of a review of the Climate Divisions across the United States. They are likely to be revised to more accurately represent areas of similar

climate. Figure 3.2 is a diagram of all the sites from which data was collected distinguished by Climate Division. Included for reference are the locations of Flagstaff, Phoenix and Tucson.

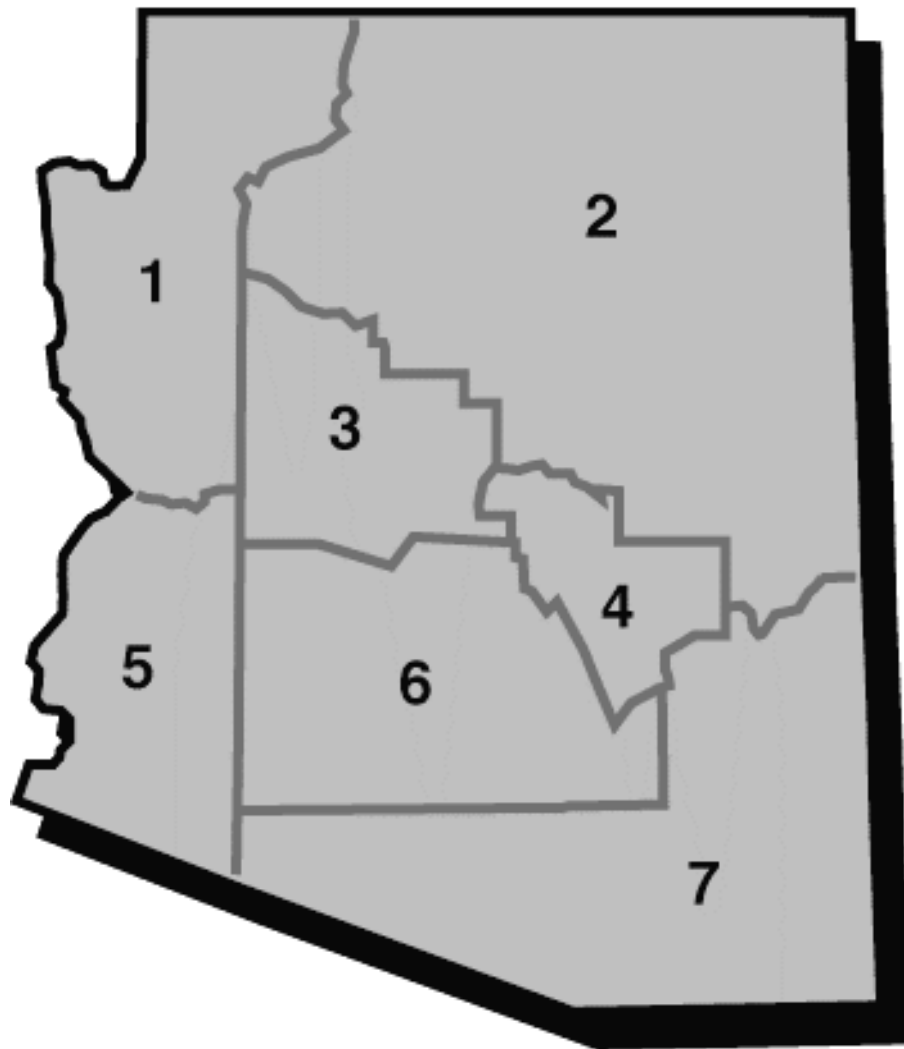


Figure 3.1  
Climate Divisions As Defined by NCDC

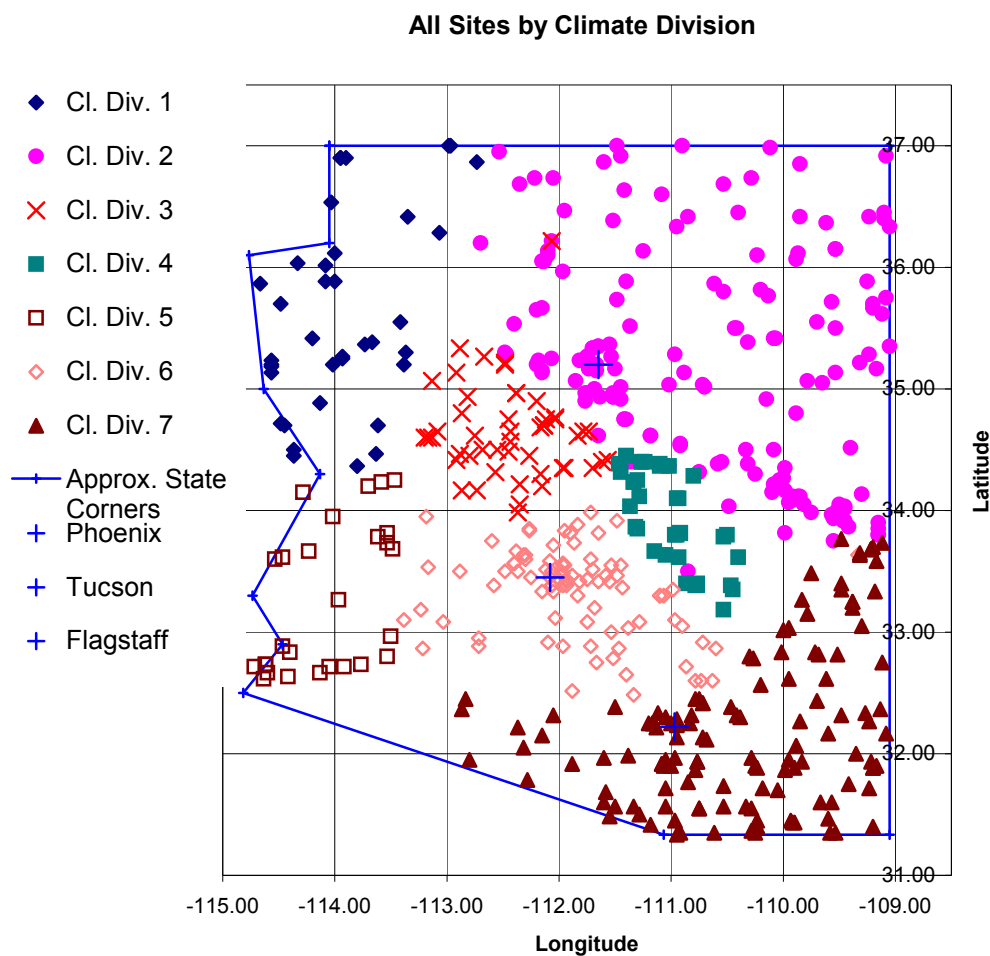


Figure 3.2  
Data Stations by Climate Divisions

Figure 3.3 shows all the sites distinguished by the source of the data. In some instances sites from different sources are at the same location. Because some sources had data for different time periods recorded each site was treated individually regardless of its proximity to other sites. Data was manipulated individually to enable statistical modeling for each site independently with independent results.

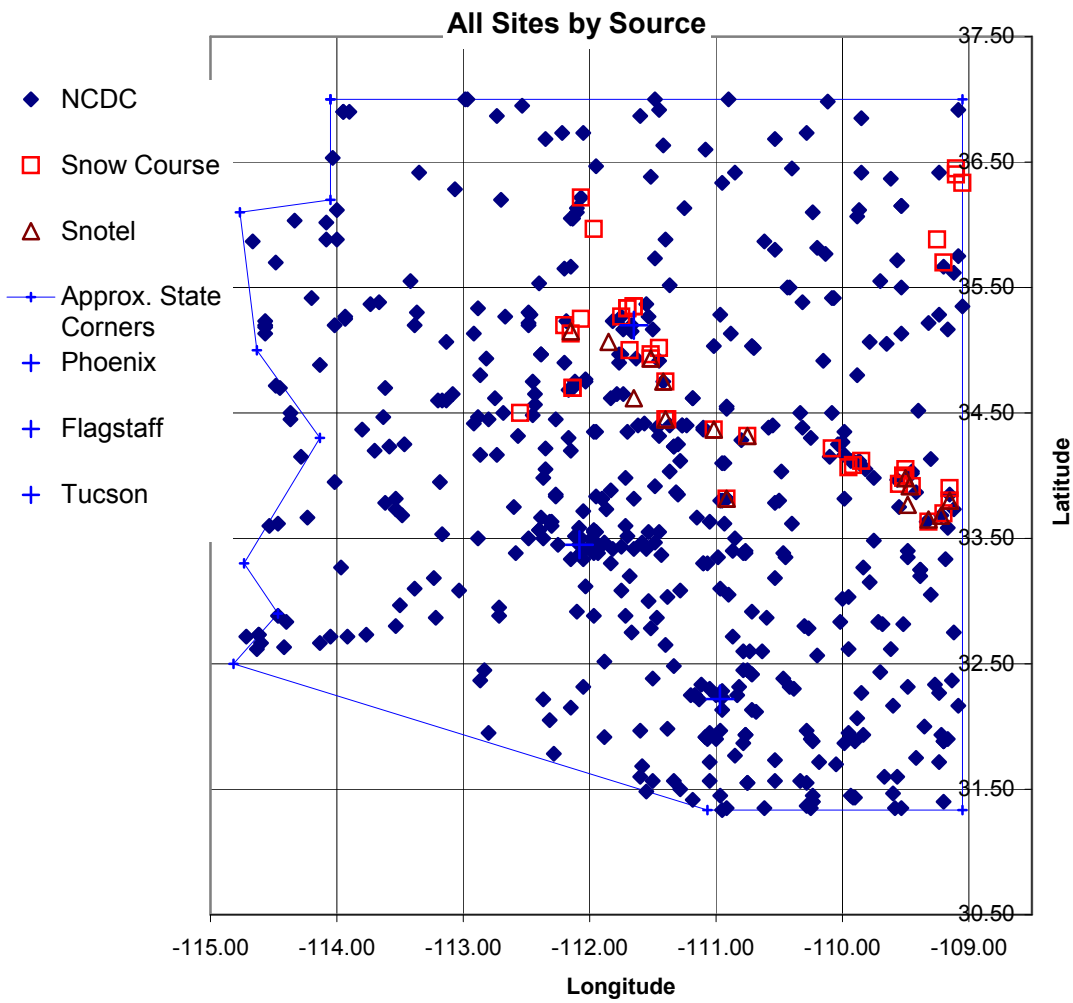


Figure 3.3  
Data Stations by Sources

### 3.2.1 NATIONAL CLIMATE DATA CENTER

The National Climate Data Center (NCDC) is a part of the United States Department of Commerce. It is specifically operated as part of the National Environmental Satellite, Data, and Information Service of the National Oceanic and Atmospheric Administration. The purpose of NCDC is to manage the Nation’s global

climatological data for monitoring, assessment, and prediction of changes in the global climate. (NCDC, 2006)

The NCDC maintains a database of information from a cooperative network of climate observers. Many different climate-monitoring organizations contribute data to the database. Information for this report was accessed from the NCDC database through the Office of the State of Arizona Climatologist. Andrew W. Ellis, Director of the Office of Climatology and State Climatologist for Arizona was contacted by email and in turn filed a request with the NCDC for the data of interest. In less than one day a response was received indicating that the data was available via an Internet .ftp site. The data remained available for seven days, after which it was deleted from the .ftp site. The data was provided for download as a zipped file containing ASCII files. There was over 150 megabytes of information in ASCII format, including historical daily snow depths for 483 individual sites in Arizona. The number of years that data was recorded varied greatly by site from 1 year to 173 years (the number of years of data is included in Table 3.4, Summary of Ground Snow Loads). In addition to the snow depth data there was miscellaneous information that was discarded (i.e. to reduce the data down before it could be manipulated for statistical modeling).

### **3.2.2 NATIONAL WEATHER AND CLIMATE CENTER**

The National Weather and Climate Center (NWCC) is part of the National Resource Conservation Service operated under the United States Department of

Agriculture. Their purpose is to promote national resource conservation by leading the development and transfer of climate information. (NWCC, 2006)

The Snow Survey program is maintained primarily to provide mountain snow pack and streamflow forecasting information. Two types of sites are maintained by the NWCC for monitoring snow water equivalents (SWE), Snow Course sites and Snotel sites.

Snow Course sites are generally meadow areas that are well protected from wind. Measurements are taken manually around the beginning and end of each month during the winter and spring. Data from these sites included the depth of snow, the SWE and the date the measurement was taken. The number of years that data was recorded varied from 6 to 68 years.

Snotel (for SNOwpack TELelemetry) sites are automated sites that transmit data via VHF radio signals to a central computer in Portland, Oregon. The SWE is measured daily by a pressure sensitive pillow. Snotel sites are generally located in high mountain watersheds. The information available from these sites included the daily SWE and the date and times of measurement. The number of years recorded for each site varied from 6 years to 23 years.

### 3.3 STATISTICAL METHODS

Consistent with the recommendations of the previously mentioned studies (see Section 2.3) and the current design load standards, a lognormal distribution was assumed for the annual extreme snow depth for NCDC sites and annual extreme SWE for the

NWCC sites. In order to verify the goodness-of-fit for the lognormal distribution a chi-square goodness-of-fit test was performed on a data set from a typical NCDC site. The McNary 2N site has records available for 73 years in which the minimum annual extreme snow depth recorded was 8 inches and the maximum annual extreme snow depth was 42 inches. The results of the test are shown in Table 3.1. Occurrences are years in which the annual extreme snow depth is within the interval indicated.

Table 3.1  
Chi-square Test Results for McNary 2N

McNary 2N			
<u>Snow Depth</u>	<u>Observed Occurrences</u>	<u>Theoretical Occurrences</u>	<u><math>(O-T)^2/T</math></u>
<10	4	4.0	0.00
10 to 15	15	16.8	0.18
15 to 19	19	16.6	0.34
19 to 25	16	18.5	0.34
20 to 29	7	7.2	0.01
30 to 39	10	7.6	0.79
>39	2	2.3	0.05
Sum =	73	73.0	1.72

With 4 degrees of freedom (7 intervals - 1 - 2 distribution parameters = 4 degrees of freedom) the resulting chi-square value is less than 9.488 corresponding to a 5% significance level, hence the lognormal distribution is acceptable at the 5% significance level. (Haldar and Mahadevan, 2000) This test was not performed on all the sites, but having a 5% significance level indicates that 5 samples out of 100 are not modeled accurately with a lognormal distribution. A 5% significance level is adequate for purposes of modeling annual extreme snow data.



After the individual data for each site was collected and reduced to annual extremes for each of the years that data was available, the mean and standard deviation for all the years of record were found for each site. From this information the Coefficient of Variation (COV) was calculated and the lognormal distribution parameters,  $\lambda$  and  $\zeta$ , were determined.

$$\text{Mean} = \mu_x = \frac{1}{n} \sum_{i=1}^n x_i \quad (3.2)$$

$$\text{Standard Deviation} = \sigma_x = \sqrt{\frac{1}{n-1} \sum_{i=1}^n (x_i - \mu_x)^2} \quad (3.3)$$

$$\text{COV} = \delta_x = \frac{\sigma_x}{\mu_x} \quad (3.4)$$

$$\lambda_x = \ln \mu_x - \frac{1}{2} \zeta_x^2 \quad (3.5)$$

$$\zeta_x^2 = \ln[1 + \delta_x^2] \quad (3.6)$$

For many of the sites, particularly those with a large number of years without snow observed, the COV was very large and artificially inflated the 50-year mean recurrence interval ground snow load value. After considering the wide variability in the COV's for some of the Arizona sites and reviewing the means and standard deviations for the 76 sites used in the Ellingwood and Redfield report (1984) (in which lognormal was designated as the best-fitting distribution), the decision was made to impose a maximum limit for the COV. If the calculated COV was greater than 1.0 the value used to

determine  $\zeta^2$  was limited to 1.0. All sites that had a calculated COV equal to or less than 1.0 were not modified. After these modifications of the COV, the results appeared more realistic and consistent than with the inflated COV's.

The snow depth values and SWE values with a 2% probability of being exceeded annually (50-year mean recurrence interval) can be determined using the lognormal parameters.

$$P(x \leq b) = \frac{1}{\sqrt{2\pi}} \int_0^{\left(\frac{\ln b - \lambda_x}{\zeta_x}\right)} \exp\left(-\frac{1}{2}s^2\right) ds \quad (3.7)$$

where  $x$  is annual maximum snow depth or annual maximum snow water equivalent

$b$  is the 50-year snow depth or 50-year snow water equivalent

$$S = \frac{\ln X - \lambda_x}{\zeta_x} \quad (3.8)$$

Alternatively:

$$P(x \leq b) = \Phi\left[\frac{\ln b - \lambda_x}{\zeta_x}\right] \quad (3.9)$$

where  $\Phi[ ]$  is the cumulative distribution function (CDF) of the standard normal distribution and is available in most texts (Haldar, 2000)

The task is to determine the value of  $b$ , or the value of  $x$  corresponding to a CDF of 0.98. A CDF = 0.98 implies a value for  $x$  with a 2% annual probability of being

exceeded ( $1-0.98 = 2\%$ ). A value for  $\left[ \frac{\ln b - \lambda_x}{\zeta_x} \right]$  that corresponds to value of

$$\Phi \left[ \frac{\ln b - \lambda_x}{\zeta_x} \right] = 1 - 0.02 = 0.98 \text{ can be found from a table commonly found in a textbook.}$$

Referencing the table in Appendix 1 of Probability, Reliability and Statistical Methods in Engineering Design (Haldar and Mahadevan, 2000) one can find that for

$$\Phi \left[ \frac{\ln b - \lambda_x}{\zeta_x} \right] = 0.98, \text{ the corresponding value of } \left[ \frac{\ln b - \lambda_x}{\zeta_x} \right] = 2.0548. \text{ Solving for } b$$

yields:

$$b = \exp[2.0548 * \zeta_x + \lambda_x] \quad (3.10)$$

This process can be repeated to determine the 30-year snow depth and snow water equivalent. For a 30-year mean recurrence interval a corresponding value for  $x$  with a  $\frac{1}{30} = 3.3\%$  annual probability of being exceeded is sought. The corresponding value of

$$\left[ \frac{\ln b - \lambda_x}{\zeta_x} \right] = 1.8339. \text{ Similar to equation 3.10 we solve for } b:$$

$$b = \exp[1.8339 * \zeta_x + \lambda_x] \quad (3.11)$$

These same steps can be applied for any mean recurrence interval of interest. The appropriate values for the CDF,  $\Phi$ , must be selected, then a table referenced for the corresponding value of  $\left[ \frac{\ln b - \lambda_x}{\zeta_x} \right]$ .

With these values the 50-year and 30-year ground snow loads were established for each site. For the sites with SWE data equation 3.1 is used to find the ground snow load. For the sites with only snow depth data the density of the snow must be determined in order to find a 50-year or 30-year ground snow load.

### 3.4 DENSITY OF SNOW

The density of snow is a difficult quantity to measure. Not only can it vary greatly from site to site, but it can also vary at a given site throughout the duration of the snow season. Furthermore, at a given site and time the density of snow in a snow pack can vary within the snow pack from the bottom of the snow pack to the top. A few reasons for this variation in density are; the packing of the snow, the wetness of the snow when it falls and the thawing and refreezing action of the snow through the season. Early in the season, when the temperatures are colder, the snow falls and remains frozen on the ground. As it accumulates the weight of the snow piling on top compresses the snow near the bottom to a higher density. Conversely, prolonged or repeated exposure to the sun can cause the snow near the top to melt down into the snow pack where it refreezes. Although the density through the depth of the snow varies, an average density of the

snow pack is considered sufficient for analysis. This average density increases as the density increases at the bottom or the top of the snow pack.

Table 3.2 illustrates the variations in snow density at two NWCC sites. It includes the measured snow depth and measured SWE at the Snowslide Canyon and Happy Jack sites for a snowstorm during March 2006.

Table 3.2  
SWE/Snow Depth Records during Storm

Snowslide Canyon					
Date	Snow Water Equivalent	Snow Depth	Change in Snow Water Equivalent	Change in Snow Depth	% Water of Snow
3/9	2.8	5.5			51%
3/10	3.3	5.7	0.5	0.2	58%
3/11	4.6	20.4	1.3	14.7	23%
3/12	6.0	34.3	1.4	13.9	17%
3/13	6.7	34.8	0.7	0.5	19%
3/14	6.6	29.4	-0.1	-5.4	22%
3/15	7.0	24.5	0.4	-4.9	29%
3/16	6.9	22.1	-0.1	-2.4	31%
Happy Jack					
Date	Snow Water Equivalent	Snow Depth	Change in Snow Water Equivalent	Change in Snow Depth	% Water of Snow
3/9	0.0	0.7			0%
3/10	0.0	0.3	0.0	-0.4	0%
3/11	0.5	3.3	0.5	3.0	15%
3/12	1.8	14.2	1.3	10.9	13%
3/13	2.3	15.0	0.5	0.8	15%
3/14	2.3	13.3	0.0	-1.7	17%
3/15	2.3	10.7	0.0	-2.6	21%
3/16	2.2	9.5	-0.1	-1.2	23%

During the week of the storm, the depth of snow drops off very quickly after the storm passes, but the SWE remains almost constant for a few days. The 5.5 inches of snow with a 51% water at the beginning of the week before the storm at Snowslide Canyon had obviously been on the ground for some time and had already experienced

significant melting. Just as new snow falling can change the density of a deep snow pack by packing it even further, the new drier snow fell and the average density decreased drastically for the Snowslide Canyon site. This same general behavior is evident at the Happy Jack site as well; on March 12 the density (or % water) decreased as the dry and light snow fell, then it melted and decreased in depth, but the weight remained consistent.

Table 3.3 compares the ground snow load measured by the SWE data and the ground snow load calculated using equation 2.1.

Table 3.3  
Load Comparison by Method

Snowslide Canyon					
Date	Snow Water Equivalent	Snow Depth	Ground Snow Load From Snow Water Equivalent	Ground Snow Load From Snow Depth (Eq. 2.1)	% Difference
3/9	2.8	5.5	14.6	2.8	81%
3/10	3.3	5.7	17.2	3.0	83%
3/11	4.6	20.4	23.9	16.9	30%
3/12	6.0	34.3	31.2	34.2	-10%
3/13	6.7	34.8	34.8	34.8	0%
3/14	6.6	29.4	34.3	27.7	19%
3/15	7.0	24.5	36.4	21.6	41%
3/16	6.9	22.1	35.9	18.8	48%
Happy Jack					
Date	Snow Water Equivalent	Snow Depth	Ground Snow Load From Snow Water Equivalent	Ground Snow Load From Snow Depth (Eq. 2.1)	% Difference
3/9	0.0	0.7	0.0	0.2	NC
3/10	0.0	0.3	0.0	0.1	NC
3/11	0.5	3.3	2.6	1.4	46%
3/12	1.8	14.2	9.4	10.3	-10%
3/13	2.3	15.0	12.0	11.1	7%
3/14	2.3	13.3	12.0	9.4	21%
3/15	2.3	10.7	12.0	7.0	41%
3/16	2.2	9.5	11.4	6.0	48%

Equation 2.1 does not give accurate results in predicting the ground snow load at these 2 sites. It does, however, predict the snow load exactly for one day of record at the Snowslide Canyon site, March 13. It would be expected that equation 2.1 would be relatively accurate; however the prediction of the daily snow load is not the intent of the study done by Tobiasson and Greatorax (1996) in the development of equation 2.1. This relationship was developed from comparing *annual* maximum snow depth to *annual* maximum SWE. This does not imply that it can be accurate for predicting snow density on any given site on any given day. It must be remember that equation 2.1 is intended to predict *annual* maximum ground snow load from *annual* maximum snow depth.

With a few exceptions, the predictions for ground snow loads shown in Table 3.3 by equation 2.1 appear to be low. This could be due to the fact that the annual extreme values for snow depth and SWE used in the development did not occur in the spring, when snow is known to have a higher density. (Fridley et al., 1994) This equation was also developed by fitting data from 204 sites across the United States to a single curve. The report does not indicate where the data for each site fell relative to the curve. There could have been areas of the U.S. that had more scatter relative to other areas. There are many possible causes for the inaccuracies in the predictions of ground snow loads, but it is impossible to come to a certain conclusion for such an isolated snowstorm incident as shown in the case study.

Although equation 2.1 cannot be expected to predict the ground snow load from snow depth for two sites in Arizona from an isolated spring snowstorm, noting the inaccuracies does cause questions to arise as to the applicability for this equation for

Arizona. To further investigate the application and suitability of this equation specifically for Arizona, a study similar to that of Tobiasson and Greatorex (1996) has been conducted.

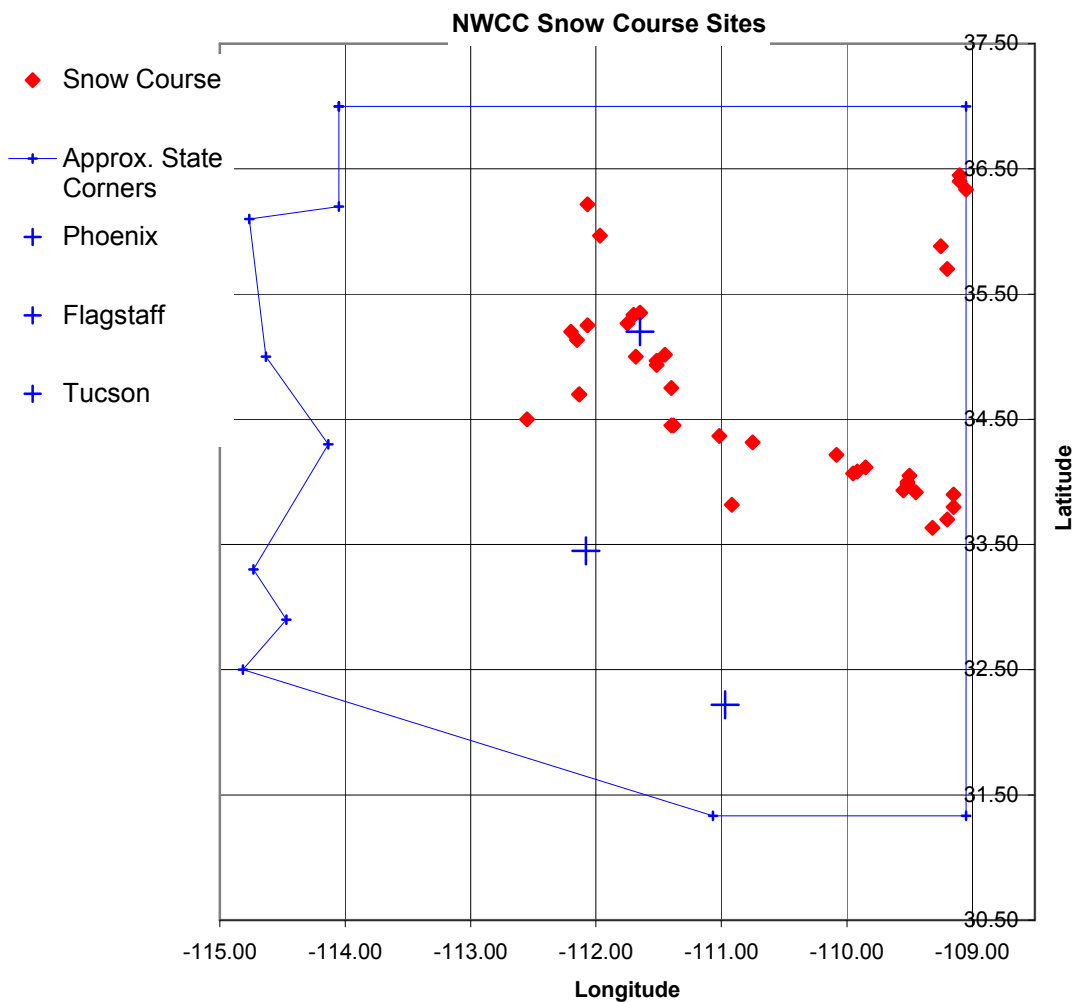


Figure 3.4  
Location of NWCC Snow Course Sites



Historical records for 41 Snow Course sites compiled for this report contain both snow depth and SWE data. Figure 3.4 shows the location of each of these sites. Comparing Figure 3.4 to Figure 3.6 its can be shown that there is a reasonable representation of those sites with ground snow loads of 12 psf or greater. For each of these sites the annual maximum snow depth and annual maximum SWE were modeled to determine a 50-year snow depth and a 50-year SWE. A lognormal distribution was assumed and the procedure presented in Chapter 3.3 was followed. The 50-year ground snow load was plotted against the 50-year snow depth to determine a density relationship. Similar to that shown in Figure 2.1, the best-fit curve was nonlinear, see Figure 3.5.

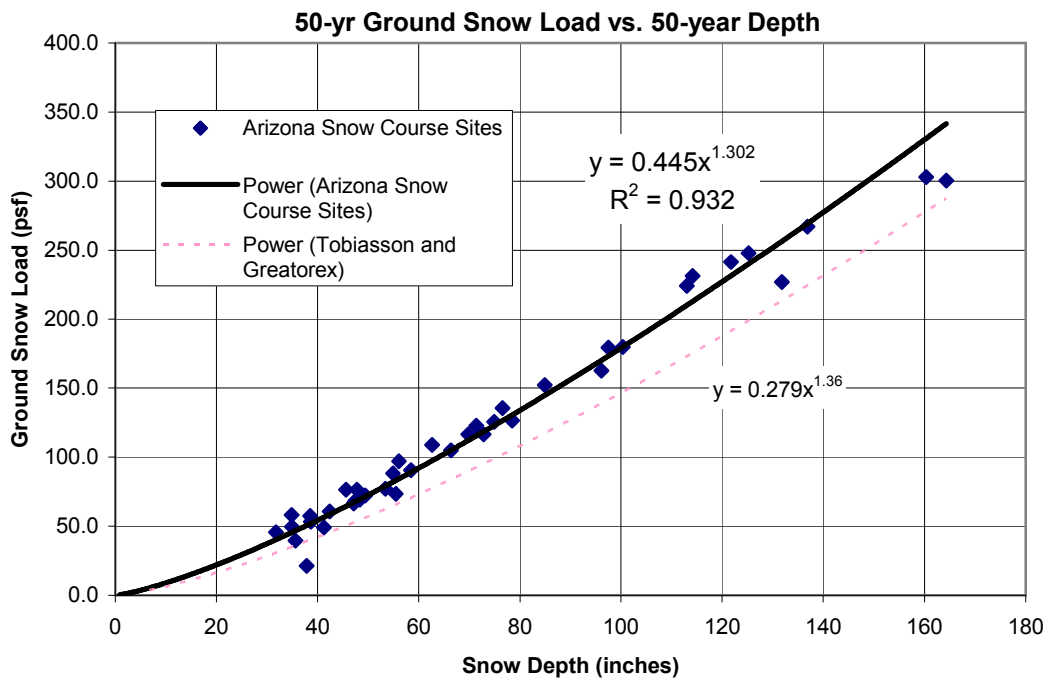


Figure 3.5  
Density Relationship for Arizona Snow Course Sites

The curve determined from Arizona sites predicts a higher load for a given depth than that determined from equation 2.1 for the entire U.S. The reasons for this may be the same as those discussed previously. The refinement of this density relationship to represent Arizona exclusively yields the equation:

$$p_g = 0.445h_g^{1.302} \quad (3.12)$$

where  $p_g$  is the ground snow load in psf (multiply by 0.048 to get kN/m<sup>2</sup>)  
 $h_g$  is the depth of snow in inches

Equation 3.12 shows only the curve that fit the scatter best, with no adjustment to increase the confidence level for the predicted ground snow load. From the Coefficient of Determination  $R^2$  value of 0.932 in Figure 3.5 it can be shown that the relationship shown in equation 3.12 is an accurate predictor accounting for 93% of the total variation in ground snow loads. (Haldar and Mahadevan, 2000) Because this curve fit the data scatter best and represents the approximate mean, if we further assume that the median is close to the mean, it can be expected that it will provide a conservative prediction approximately half of the time.

Because equation 3.12 fits the data available for Arizona better than equation 2.1, it is used in this study for determining ground snow loads from snow depth; nevertheless, determining the density of snow is a complex task with numerous random variables and has been the source of many inaccuracies for decades. (Elliott, 1981)

### 3.5 GROUND SNOW LOADS

As expected, many sites at lower elevations throughout the state have 50-year Ground Snow Loads equal to zero. All sites in Climate Division 5 had a calculated ground snow load equal to zero. Figure 3.6 shows the location of all sites with a 50-year Ground Snow Load of 12 pounds per square foot (psf) or greater.

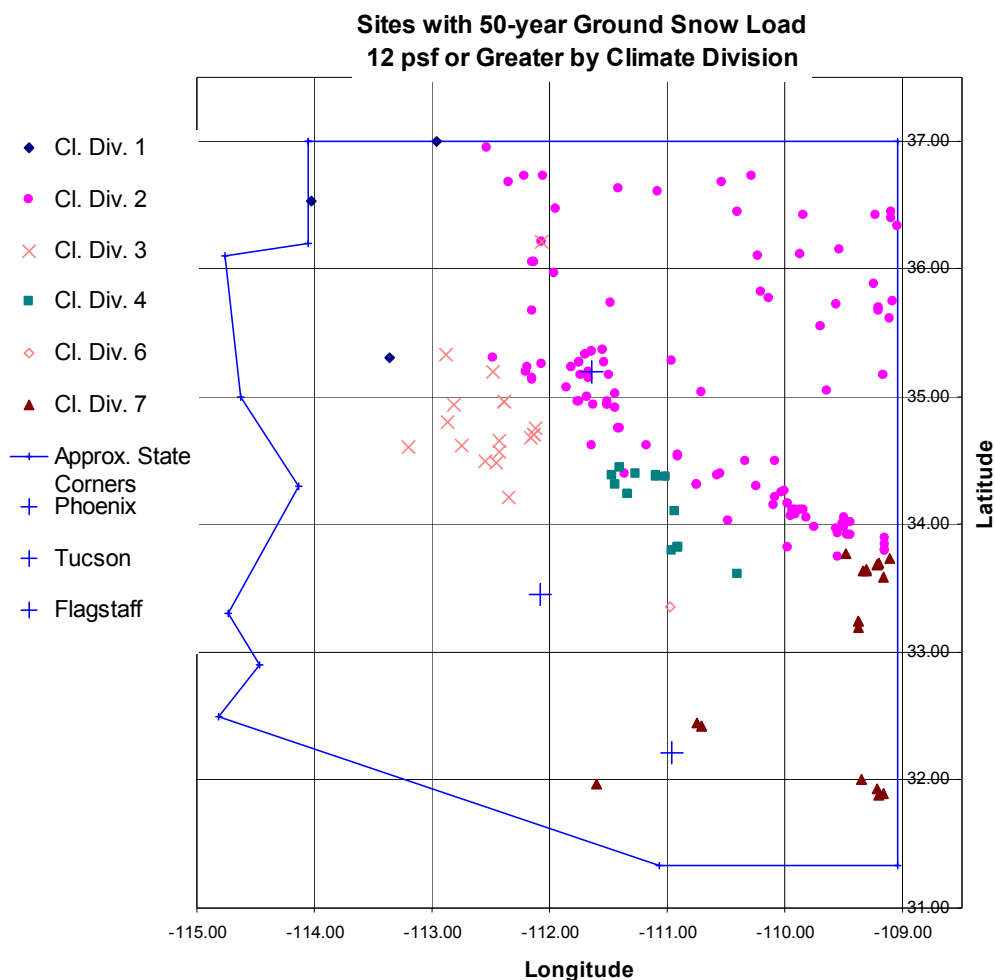


Figure 3.6  
Data Stations with 50-year Ground Snow Loads Equal to 12 psf or Greater

The majority (63.6%) of the sites that are represented by having 50-year ground snow loads greater than 11 psf fall in Climate Division 2. Climate Divisions 1 (1.7%), 3 (10.2%), 4 (9.7%), 6 (0.6%) and 7 (11.4%) make up the remaining representation of sites with ground snow loads equal to 12 psf or greater.

All of the ground snow loads determined in this study are presented in Table 3.4. Some locations had records for more than one site (town, city, etc. is meant by *location*). During statistical modeling each site was kept independent and modeled individually. For the purposes of summarizing the data, only one site was listed for each location, with a few exceptions. For locations with more than one site at elevations differing by more than 300 feet (91 meters) and having a significantly different snow load calculated for each, both sites are shown in the table. For locations where one or more site records are not included in the table, three criteria were used to determine which site record and ground snow load value would be shown. 1) If a site had more than 15 years of data included in determining the ground snow load it was given precedence. 2) Those sites that had the ground snow loads determined by SWE data were considered to have more reliable records than site with only snow depth data (due to the error inherent in converting snow depth to ground snow load discussed in Section 3.4). 3) If considering the two preceding items could not determine which site was more accurate, the Coefficient of Variation values were compared, the site that had more consistent annual maxima (a lower COV) was given priority. These rules were not hard and fast rules, each location with multiple sites was considered individually and if, after the application of the

3 listed criteria the preferred site could still not be obtained, the record with a more conservative ground snow load was chosen.

Station Name	County	Climate Division	Longitude	Latitude	Elev. (ft (m))	Max Snow Depth (in)	Max SWE (in of H <sub>2</sub> O)	Year of Max	Month of Max	# of Years of Data	50-yr Ground Snow Load (psf)	30-yr Ground Snow Load (psf)	Data Source
* BEAVER DAM	MOHAVE	1	-113.95	36.90	1875 (572)	--	--	--	--	1	0	0	NCDC
BEAVER DAM	MOHAVE	1	-113.95	36.90	1875 (572)	--	--	--	--	43	0	0	NCDC
BULLHEAD CITY	MOHAVE	1	-114.57	35.13	540 (165)	--	--	--	--	29	0	0	NCDC
* CHLORIDE	MOHAVE	1	-114.20	35.42	4021 (1226)	--	--	--	--	1	0	0	NCDC
COLORADO CITY	MOHAVE	1	-112.97	37.00	5009 (1527)	13	--	1979	2	43	10	8	NCDC
* COLORADO CITY	MOHAVE	1	-112.98	37.00	5012 (1528)	8	--	1951	4	13	8	6	NCDC
* DAVIS DAM	MOHAVE	1	-114.57	35.18	531 (162)	--	--	--	--	11	0	0	NCDC
DAVIS DAM # 2	MOHAVE	1	-114.57	35.20	659 (201)	--	--	--	--	20	0	0	NCDC
* DIAMOND BAR RANCH	MOHAVE	1	-114.00	35.88	3601 (1098)	--	--	--	--	5	0	0	NCDC
* DIAMOND M RANCH	MOHAVE	1	-113.37	35.30	5479 (1670)	15	--	2001	1	7	19	15	NCDC
* HACKBERRY	MOHAVE	1	-113.73	35.37	3582 (1092)	--	--	--	--	5	0	0	NCDC
* KATHERINE RANGER STN	MOHAVE	1	-114.57	35.23	670 (204)	--	--	--	--	2	0	0	NCDC
KINGMAN AAF	MOHAVE	1	-113.93	35.27	3385 (1032)	7	--	1922	3	67	2	1	NCDC
* KINGMAN AIRPORT	MOHAVE	1	-113.93	35.25	3419 (1042)	--	--	--	--	3	0	0	NCDC
KINGMAN NO 2	MOHAVE	1	-114.02	35.20	3538 (1079)	--	--	--	--	27	0	0	NCDC
LAKE HAVASU	MOHAVE	1	-114.37	34.45	482 (147)	--	--	--	--	25	0	0	NCDC
LAKE HAVASU CITY	MOHAVE	1	-114.37	34.50	468 (143)	--	--	--	--	15	0	0	NCDC
* LITTLEFIELD 1 NE	MOHAVE	1	-113.90	36.90	1904 (581)	--	--	--	--	12	0	0	NCDC
* LITTLEFIELD 25 SSW	MOHAVE	1	-114.03	36.53	4002 (1220)	12	--	1949	1	5	9	7	NCDC
* LOOKOUT RANCH	MOHAVE	1	-113.38	35.20	5002 (1525)	--	--	--	--	7	0	0	NCDC
* MEADVIEW 1SE	MOHAVE	1	-114.08	36.02	3199 (975)	--	--	--	--	10	0	0	NCDC
MOUNT TRUMBULL	MOHAVE	1	-113.35	36.42	5603 (1708)	40	--	1945	12	57	8	6	NCDC
PEACH SPRINGS	MOHAVE	1	-113.42	35.55	4969 (1515)	27	--	1967	12	56	4	3	NCDC
* PIERCE FERRY	MOHAVE	1	-114.00	36.12	1371 (418)	--	--	--	--	5	0	0	NCDC
PIERCE FERRY 17 SSW	MOHAVE	1	-114.08	35.88	3857 (1176)	11	--	1979	2	22	6	4	NCDC
PIPE SPRINGS NATL MON	MOHAVE	1	-112.73	36.87	4919 (1500)	18	--	1973	1	43	5	4	NCDC
* SIGNAL	MOHAVE	1	-113.63	34.47	1522 (464)	--	--	--	--	1	0	0	NCDC
* SIGNAL 13 SW	MOHAVE	1	-113.80	34.37	2512 (766)	--	--	--	--	9	0	0	NCDC
TEMPLE BAR	MOHAVE	1	-114.33	36.03	1280 (390)	--	--	--	--	19	0	0	NCDC
* TOPOCK	MOHAVE	1	-114.48	34.72	449 (137)	--	--	--	--	3	0	0	NCDC
* TOPOCK 2 SSE	MOHAVE	1	-114.45	34.70	502 (153)	--	--	--	--	13	0	0	NCDC
TRUXTON CANYON	MOHAVE	1	-113.67	35.38	3819 (1164)	12	--	1932	12	54	4	3	NCDC
TUWEEP	MOHAVE	1	-113.07	36.28	4774 (1455)	15	--	1967	12	45	7	5	NCDC
* WHITE HILLS 5 WSW	MOHAVE	1	-114.48	35.70	2430 (741)	--	--	--	--	4	0	0	NCDC
WIKIEUP	MOHAVE	1	-113.62	34.70	2009 (613)	--	--	--	--	54	0	0	NCDC
WILLOW BEACH	MOHAVE	1	-114.67	35.87	740 (226)	--	--	--	--	39	0	0	NCDC
YUCCA 1 NNE	MOHAVE	1	-114.13	34.88	1950 (594)	--	--	--	--	53	0	0	NCDC
ALPINE	APACHE	2	-109.15	33.85	8048 (2454)	60	--	1967	12	97	38	31	NCDC
ARBABS FOREST	APACHE	2	-109.20	35.70	7680 (2341)	--	6	98	2	21	41	36	S. Course
* ASH FORK 5 N	COCONINO	2	-112.47	35.28	5325 (1623)	8	--	1978	3	8	6	6	NCDC
ASH FORK 6 N	COCONINO	2	-112.48	35.30	5304 (1617)	38	--	1967	12	71	12	9	NCDC
Baldy	Apache	2	-109.50	33.98	9125 (2782)	--	17.6	83	4	23	115	103	Snotel
BALDY #1 - SNOW COURSE AND AER	APACHE	2	-109.52	33.98	9125 (2782)	--	18.3	79	3	50	110	99	S. Course
BALDY #2	APACHE	2	-109.55	33.93	9750 (2973)	--	41.8	79	3	36	238	214	S. Course
BEAR PAW	COCONINO	2	-111.65	35.35	10100 (3079)	--	51	93	E/ST	37	271	244	S. Course
BEAVER SPRING	APACHE	2	-109.05	36.33	9220 (2811)	--	16.8	93	3	20	103	96	S. Course
* BELLEMONT NWFO	COCONINO	2	-111.82	35.23	7150 (2180)	20	--	2005	1	7	26	22	NCDC
BETATAKIN	NAVAJO	2	-110.53	36.68	7284 (2221)	38	--	1967	12	67	26	23	NCDC
* BIG LAKE	NAVAJO	2	-109.42	33.87	9004 (2745)	--	--	--	--	2	0	0	NCDC
* BITA HOCHEE TRADING POST	NAVAJO	2	-110.08	35.42	5904 (1800)	--	--	--	--	8	0	0	NCDC
* BLACK MOUNTAIN MISSION	APACHE	2	-109.87	36.12	6353 (1937)	12	--	1961	12	9	10	8	NCDC
BLUE RIDGE RANGER STN	COCONINO	2	-111.18	34.62	6878 (2097)	38	--	1997	1	39	39	31	NCDC
BRIGHT ANGEL RS	COCONINO	2	-112.07	36.22	8398 (2560)	89	--	1993	2	81	164	138	NCDC
* BUCK SPRING	APACHE	2	-109.85	34.12	7400 (2256)	--	5.4	98	2	8	35	30	S. Course
* BUFFALO RANCH	COCONINO	2	-111.95	36.47	5662 (1726)	21	--	1960	12	4	32	25	NCDC
BURRUS RANCH	COCONINO	2	-111.53	35.27	6802 (2074)	70	--	1949	2	25	67	54	NCDC
CAMERON 1 NNE	COCONINO	2	-111.40	35.88	4163 (1269)	18	--	1967	12	31	2	1	NCDC
* CAMP GERONIMO	GILA	2	-111.37	34.40	5514 (1681)	15	--	1978	2	2	28	22	NCDC
CANYON DE CHELLE	APACHE	2	-109.53	36.15	5608 (1710)	8	--	1974	1	36	2	2	NCDC
* CEDAR RIDGE TRADING POST	COCONINO	2	-111.52	36.38	5923 (1806)	--	--	--	--	6	0	0	NCDC
CHALENDER	COCONINO	2	-112.07	35.25	7100 (2165)	--	12	73	3	59	63	55	S. Course
CHEESE SPRINGS	APACHE	2	-109.50	34.05	8700 (2652)	--	12.4	73	3	37	72	66	S. Course
CHEVELON RS	COCONINO	2	-110.92	34.55	7004 (2135)	52	--	1967	12	45	43	35	NCDC
CHINLE	APACHE	2	-109.53	36.15	5544 (1690)	14	--	1967	12	57	9	7	NCDC
CIBECUE	NAVAJO	2	-110.48	34.03	4979 (1518)	18	--	1937	1	52	14	11	NCDC
CLAY SPRINGS	NAVAJO	2	-110.32	34.38	6318 (1926)	--	--	--	--	16	0	0	NCDC
COPPER MINE TRADING POST	COCONINO	2	-111.42	36.63	6383 (1946)	23	--	1960	12	38	13	10	NCDC
Coronado Trail	Apache	2	-109.15	33.80	8400 (2561)	--	13.8	93	3	23	75	66	Snotel
CORONADO TRAIL	APACHE	2	-109.15	33.80	8350 (2546)	--	12.5	49	2	68	65	57	S. Course

Station Name	County	Climate Division	Longitude	Latitude	Elev. (ft (m))	Max Snow Depth (in)	Max SWE (in of H <sub>2</sub> O)	Year of Max	Month of Max	# of Years of Data	50-yr Ground Snow Load (psf)	30-yr Ground Snow Load (psf)	Data Source
FORT APACHE	APACHE	2	-109.52	34.00	9160 (2793)	--	17.6	93	3	55	107	96	S. Course
FORT DEFIANCE		2	-109.08	35.75	6904 (2105)	14	--	1903	2	19	13	10	NCDC
FORT VALLEY	COCONINO	2	-111.75	35.27	7345 (2239)	57	--	1949	1	97	73	60	NCDC
FORT VALLEY	COCONINO	2	-111.75	35.27	7350 (2241)	--	11.6	49	2	59	60	52	S. Course
FREDONIA	COCONINO	2	-112.53	36.95	4681 (1427)	18	--	1973	1	41	11	9	NCDC
Fry	Coconino	2	-111.85	35.07	7200 (2195)	--	15	93	3	23	95	86	Snotel
GANADO	APACHE	2	-109.57	35.72	6338 (1932)	24	--	1967	12	77	12	10	NCDC
GRAND CANYON	COCONINO	2	-111.97	35.97	7500 (2287)	--	10.6	73	3	59	52	45	S. Course
GRAND CANYON AIRWAYS	COCONINO	2	-112.13	36.05	6973 (2126)	40	--	1949	1	20	38	31	NCDC
GRAND CANYON HDQS		2	-112.13	36.05	6891 (2101)	38	--	1949	1	54	38	32	NCDC
GRAND CANYON N P 2	COCONINO	2	-112.15	36.05	6783 (2068)	28	--	1979	2	30	25	22	NCDC
GRAND CANYON NATL PARK	COCONINO	2	-112.13	36.05	6953 (2120)	29	--	1967	12	21	30	27	NCDC
*GRAY MOUNTAIN TRADING POST		2	-111.48	35.73	4914 (1498)	15	--	1962	1	7	14	11	NCDC
*GREER	APACHE	2	-109.45	34.02	8273 (2522)	54	--	1967	12	81	61	50	NCDC
*GREER LAKES	APACHE	2	-109.45	34.03	8505 (2593)	--	--	--	--	1	0	0	NCDC
*HAPPY JACK	COCONINO	2	-111.40	34.75	7630 (2326)	--	20.1	73	3	55	90	79	S. Course
*Happy Jack	Coconino	2	-111.42	34.75	7630 (2326)	--	11.6	05	3	6	78	69	Snotel
*HAPPY JACK RS	COCONINO	2	-111.42	34.75	7478 (2280)	58	--	1997	1	37	76	63	NCDC
HAWLEY LAKE	APACHE	2	-109.75	33.98	8178 (2493)	91	--	1967	12	22	146	132	NCDC
HEBER	COCONINO	2	-110.75	34.32	7640 (2329)	--	16.2	73	3	50	77	68	S. Course
Heber	Coconino	2	-110.75	34.32	7640 (2329)	--	11.6	98	3	23	75	68	Snotel
*HEBER		2	-110.58	34.38	6504 (1983)	30	--	1922	2	14	45	36	NCDC
HEBER (BLACK MESA) RANGER STA	NAVAJO	2	-110.55	34.40	6588 (2009)	48	--	1967	12	56	35	29	NCDC
HOLBROOK	NAVAJO	2	-110.15	34.92	5084 (1550)	19	--	1967	12	111	5	4	NCDC
*HOUCK 2 W		2	-109.23	35.28	5812 (1772)	--	--	--	--	1	0	0	NCDC
*HOUSE ROCK		2	-112.05	36.73	5382 (1641)	15	--	1948	12	7	11	8	NCDC
*INDIAN WELLS TRADING POST	NAVAJO	2	-110.07	35.42	5904 (1800)	--	--	--	--	1	0	0	NCDC
INNER CANYON USGS	COCONINO	2	-112.10	36.10	2571 (784)	--	--	--	--	21	0	0	NCDC
JACOB LAKE	COCONINO	2	-112.22	36.73	7823 (2385)	60	--	1973	4	32	77	64	NCDC
JEDDITO		2	-110.13	35.77	6704 (2044)	16	--	1937	1	25	14	11	NCDC
JUNIPINE	COCONINO	2	-111.75	34.97	5134 (1565)	44	--	1937	1	47	39	31	NCDC
*KAIBITO		2	-111.08	36.60	6002 (1830)	14	--	1960	12	11	16	13	NCDC
KAYENTA	NAVAJO	2	-110.28	36.73	5704 (1739)	24	--	1915	12	64	11	9	NCDC
*KAYENTA 21 SSW	NAVAJO	2	-110.40	36.45	6524 (1989)	12	--	1974	1	4	18	15	NCDC
KEAMS CANYON	NAVAJO	2	-110.20	35.82	6203 (1891)	12	--	1906	11	69	10	8	NCDC
*KLAGETOH		2	-109.53	35.50	6402 (1952)	7	--	1952	11	9	5	4	NCDC
KLAGETOH 12 WNW	APACHE	2	-109.70	35.55	6498 (1981)	36	--	1967	12	35	16	12	NCDC
LAKE MARY	COCONINO	2	-111.45	35.02	6930 (2113)	--	7.8	79	2	31	52	46	S. Course
LAKESIDE RANGER STN	NAVAJO	2	-109.98	34.17	6704 (2044)	52	--	1967	12	44	52	43	NCDC
LEES FERRY	COCONINO	2	-111.60	36.87	3209 (978)	8	--	1967	12	84	1	0	NCDC
LEUPP	COCONINO	2	-110.97	35.28	4704 (1434)	55	--	1918	12	50	12	9	NCDC
LUKACHUKAI	APACHE	2	-109.23	36.42	6518 (1987)	20	--	1961	12	59	11	9	NCDC
*LUPTON		2	-109.05	35.35	6212 (1894)	--	--	--	--	3	0	0	NCDC
MANY FARMS SCHOOL	APACHE	2	-109.62	36.37	5314 (1620)	13	--	1961	12	25	3	3	NCDC
MAVERICK	APACHE	2	-109.55	33.75	7803 (2379)	42	--	1961	12	20	61	53	NCDC
Maverick Fork	Apache	2	-109.47	33.92	9200 (2805)	--	23.6	93	3	23	139	125	Snotel
MAVERICK FORK SNOW COURSE & A	APACHE	2	-109.45	33.92	9150 (2790)	--	23.5	93	E/ST	50	144	128	S. Course
MC NARY 2 N	NAVAJO	2	-109.87	34.12	7338 (2237)	42	--	1967	12	73	45	40	NCDC
MCNARY (DISC.)	NAVAJO	2	-109.92	34.08	7200 (2195)	--	11.6	73	3	51	61	54	S. Course
*METEOR CRATER	COCONINO	2	-111.02	35.03	5662 (1726)	--	--	--	--	6	0	0	NCDC
MILK RANCH (DISC.)	NAVAJO	2	-109.95	34.07	7000 (2134)	--	8	73	3	49	43	38	S. Course
MONUMENT VALLEY	NAVAJO	2	-110.12	36.98	5563 (1696)	8	--	1989	2	26	2	2	NCDC
MORMON LAKE RNGR STN	COCONINO	2	-111.45	34.92	7183 (2190)	20	--	1963	1	30	11	9	NCDC
Mormon Mountain	Coconino	2	-111.52	34.93	7500 (2287)	--	17	93	3	23	105	93	Snotel
MORMON MOUNTAIN - SNOW COURSE	COCONINO	2	-111.52	34.93	7500 (2287)	--	24.1	73	3	50	110	97	S. Course
MORMON MOUNTAIN SUMMIT #2	COCONINO	2	-111.52	34.97	8470 (2582)	--	33.6	93	2	31	202	181	S. Course
MUND'S PARK	COCONINO	2	-111.63	34.93	6468 (1972)	22	--	1990	2	20	29	26	NCDC
NAVAJO	APACHE	2	-109.53	35.13	5583 (1702)	--	--	--	--	16	0	0	NCDC
NEWMAN PARK	COCONINO	2	-111.68	35.00	6750 (2058)	--	14.4	73	3	43	63	55	S. Course
NUTRIOSO	APACHE	2	-109.15	33.90	8500 (2591)	--	9.3	49	2	68	43	38	S. Course
OAK CREEK CANYON	COCONINO	2	-111.77	34.97	5074 (1547)	19	--	1999	4	24	19	15	NCDC
*ORAIBI	NAVAJO	2	-110.62	35.87	5934 (1809)	--	--	--	--	13	0	0	NCDC
PAGE	COCONINO	2	-111.45	36.92	4269 (1302)	9	--	1967	12	47	5	4	NCDC
PAINTED DESERT N P	APACHE	2	-109.78	35.07	5758 (1756)	11	--	1976	11	33	8	6	NCDC
PETRIFIED FOREST N P	NAVAJO	2	-109.88	34.80	5444 (1660)	14	--	1967	12	75	7	6	NCDC
PHANTOM RANCH	COCONINO	2	-112.10	36.13	2529 (771)	8	--	1971	1	40	0	0	NCDC
PINEDALE	NAVAJO	2	-110.25	34.30	6504 (1983)	42	--	1937	1	57	46	37	NCDC
PINETOP	NAVAJO	2	-109.93	34.12	6958 (2121)	31	--	1987	2	18	27	24	NCDC
PINETOP 2E	NAVAJO	2	-109.92	34.12	7198 (2195)	54	--	1967	12	63	46	39	NCDC

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SANDERS	APACHE	2	-109.32	35.22	5852 (1784)	14	--	1967	12	49	8	6	NCDC
SANDERS 11 ESE	APACHE	2	-109.17	35.17	6248 (1905)	20	--	1967	12	26	14	11	NCDC
* SEBA DALKAI SCHOOL	NAVAJO	2	-110.43	35.50	5904 (1800)	--	--	--	--	10	0	0	NCDC
SEDONA	COCONINO	2	-111.77	34.90	4219 (1286)	16	--	1949	1	62	3	3	NCDC
* SHONGOPOVI		2	-110.53	35.80	6123 (1867)	--	--	--	--	2	0	0	NCDC
SHOW LOW	NAVAJO	2	-110.00	34.27	6412 (1955)	20	--	1905	4	39	22	18	NCDC
SHOW LOW CITY	NAVAJO	2	-110.03	34.25	6438 (1963)	41	--	1967	12	40	21	17	NCDC
SILVER CREEK RANCH	NAVAJO	2	-109.98	34.35	6183 (1876)	14	--	1952	11	20	7	5	NCDC
SNOW BOWL #1 ALT.	COCONINO	2	-111.70	35.33	9920 (3024)	--	40.2	93	3	22	204	182	S. Course
SNOW BOWL #2	COCONINO	2	-111.70	35.33	11200 (3415)	--	51.6	93	3	41	272	246	S. Course
SNOWFLAKE	NAVAJO	2	-110.08	34.50	5641 (1720)	30	--	1967	12	106	11	8	NCDC
SNOWFLAKE 15 W	NAVAJO	2	-110.33	34.50	6078 (1853)	24	--	1967	12	34	17	13	NCDC
* Snowslide Canyon	Coconino	2	-111.65	34.62	9730 (2966)	--	41	05	4	8	246	222	Snotel
SNOWSLIDE CANYON	COCONINO	2	-111.65	35.35	9750 (2973)	--	47.5	93	E/ST	37	218	194	S. Course
SPRINGERVILLE	APACHE	2	-109.30	34.13	7035 (2145)	14	--	1961	11	95	8	6	NCDC
ST MICHAELS 6 WNW	APACHE	2	-109.20	35.67	7642 (2330)	14	--	1913	2	22	13	11	NCDC
SUNRISE MOUNTAIN	APACHE	2	-109.57	33.97	9368 (2856)	85	--	1973	4	16	173	135	NCDC
SUNSET CRATER NATL MONUMENT	COCONINO	2	-111.55	35.37	6978 (2128)	31	--	1973	3	37	31	27	NCDC
SUPAI	COCONINO	2	-112.70	36.20	3203 (977)	--	--	--	--	44	0	0	NCDC
TEEC NOS POS	APACHE	2	-109.08	36.92	5289 (1612)	10	--	1974	1	44	4	3	NCDC
* TEES TO		2	-110.42	35.50	5803 (1769)	7	--	1959	12	8	1	1	NCDC
* TIMBER RANGER STN	COCONINO	2	-111.18	34.62	6812 (2077)	--	--	--	--	3	0	0	NCDC
* TONALEA		2	-110.95	36.33	5514 (1681)	--	--	--	--	2	0	0	NCDC
ITSAILE CANYON #1	APACHE	2	-109.10	36.40	8160 (2488)	--	11.2	95	3	21	73	67	S. Course
ITSAILE CANYON #3	APACHE	2	-109.10	36.45	8920 (2720)	--	15.4	93	2	20	93	87	S. Course
TUBA CITY	COCONINO	2	-111.25	36.13	4987 (1520)	20	--	1967	12	98	5	4	NCDC
* VALLE	COCONINO	2	-112.20	35.65	5891 (1796)	--	--	--	--	2	0	0	NCDC
* VALLE AIRPORT		2	-112.15	35.67	6002 (1830)	10	--	1949	1	12	11	9	NCDC
WAHWEAP	COCONINO	2	-111.48	37.00	3729 (1137)	8	--	1967	12	45	1	1	NCDC
WALLACE RANGER STN		2	-110.92	34.53	7012 (2138)	30	--	1949	1	41	33	27	NCDC
WALNUT CANYON NATL MONUMENT	COCONINO	2	-111.50	35.17	6683 (2038)	54	--	1967	12	57	36	30	NCDC
WHITE HORSE LAKE JCT	COCONINO	2	-112.15	35.13	7180 (2189)	--	16.6	73	3	33	79	69	S. Course
Whitehorse Lake	Coconino	2	-112.15	35.15	7180 (2189)	--	13.5	93	3	23	83	74	Snotel
WHITERIVER 1 SW	NAVAJO	2	-109.98	33.82	5119 (1561)	21	--	1960	1	101	12	9	NCDC
WILLIAMS	COCONINO	2	-112.18	35.23	6748 (2057)	52	--	1949	1	106	44	36	NCDC
* WILLIAMS 24 NWW	COCONINO	2	-112.40	35.53	5753 (1754)	--	--	--	--	4	0	0	NCDC
WILLIAMS CREEK FISH HATCHERY	APACHE	2	-109.82	34.05	6963 (2123)	38	--	1967	12	15	46	38	NCDC
WILLIAMS SKI RUN	COCONINO	2	-112.20	35.20	7720 (2354)	--	24.6	73	4	39	136	122	S. Course
WINDOW ROCK 4 SW	APACHE	2	-109.12	35.62	6918 (2109)	18	--	1967	12	62	13	10	NCDC
* WINSLOW	NAVAJO	2	-110.70	35.02	4868 (1484)	--	--	--	--	2	0	0	NCDC
WINSLOW MUNICIPAL AP	NAVAJO	2	-110.72	35.03	4885 (1489)	29	--	1967	12	89	10	8	NCDC
WUPATKI NM	COCONINO	2	-111.37	35.52	4907 (1496)	32	--	1967	12	63	7	5	NCDC
* ANVIL ROCK		3	-113.13	35.07	5261 (1604)	--	--	--	--	2	0	0	NCDC
* ASH FORK 12 WNW	YAVAPAI	3	-112.67	35.27	5739 (1750)	9	--	1988	1	8	8	7	NCDC
ASH FORK 3	YAVAPAI	3	-112.48	35.20	5074 (1547)	12	--	1990	1	17	8	7	NCDC
* ASH FORK CAMPGROUND	YAVAPAI	3	-112.48	35.22	5150 (1570)	--	--	--	--	1	0	0	NCDC
* BAGDAD		3	-113.20	34.60	3201 (976)	17	--	1927	2	9	13	10	NCDC
BAGDAD	YAVAPAI	3	-113.17	34.60	3954 (1206)	10	--	1932	12	73	1	1	NCDC
* BAGDAD 2	YAVAPAI	3	-113.13	34.60	4116 (1255)	--	--	--	--	9	0	0	NCDC
BAGDAD 8 NE	YAVAPAI	3	-113.08	34.65	4241 (1293)	15	--	1967	12	26	6	5	NCDC
BEAVER CREEK	YAVAPAI	3	-111.78	34.65	3524 (1074)	6	--	1965	4	50	1	1	NCDC
BRIGHT ANGEL	COCONINO	3	-112.07	36.22	8400 (2561)	--	27.9	93	3	59	154	135	S. Course
BUMBLE BEE	YAVAPAI	3	-112.15	34.20	2502 (763)	11	--	1967	12	28	1	1	NCDC
CAMP WOOD	YAVAPAI	3	-112.87	34.80	5713 (1742)	36	--	1967	12	31	18	14	NCDC
CASTLE HOT SPRINGS	YAVAPAI	3	-112.37	33.98	1990 (607)	--	--	--	--	48	0	0	NCDC
* CASTLE HOT SPRINGS 4 N		3	-112.35	34.05	2801 (854)	--	--	--	--	10	0	0	NCDC
CEDAR GLADE		3	-112.38	34.97	4651 (1418)	28	--	1930	1	40	18	14	NCDC
CHILDS	YAVAPAI	3	-111.70	34.35	2649 (808)	10	--	1937	1	91	0	0	NCDC
CHINO VALLEY	YAVAPAI	3	-112.45	34.75	4749 (1448)	22	--	1967	12	65	6	4	NCDC
* CONGRESS	YAVAPAI	3	-112.87	34.17	3021 (921)	--	--	--	--	11	0	0	NCDC
COPPER BASIN DIVIDE	YAVAPAI	3	-112.55	34.50	6720 (2049)	--	11.3	68	12	35	57	49	S. Course
CORDES	YAVAPAI	3	-112.17	34.30	3770 (1149)	15	--	1967	12	74	2	1	NCDC
CROWN KING	YAVAPAI	3	-112.35	34.22	5918 (1804)	54	--	1967	12	82	55	43	NCDC
* DRAKE RANGER STN		3	-112.38	34.97	4651 (1418)	12	--	1961	12	9	13	10	NCDC
* DUGAS 2 SE	YAVAPAI	3	-111.95	34.35	4041 (1232)	--	--	--	--	13	0	0	NCDC
FOSSIL SPRINGS	YAVAPAI	3	-111.57	34.42	4271 (1302)	--	--	--	--	20	0	0	NCDC
GADDES CANYON	YAVAPAI	3	-112.13	34.70	7600 (2317)	--	20.8	73	3	36	107	93	S. Course
GROOM CREEK	YAVAPAI	3	-112.45	34.48	6104 (1861)	36	--	1949	1	32	35	29	NCDC
* HILLSIDE		3	-112.92	34.42	3851 (1174)	10	--	1949	1	8	3	2	NCDC



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SELIGMAN	YAVAPAI	3	-112.88	35.33	5249 (1600)	16	--	1906	1	101	9	7	NCDC
SELIGMAN 13 SSW	YAVAPAI	3	-112.92	35.13	5242 (1598)	--	--	--	--	20	0	0	NCDC
* SKULL VALLEY	YAVAPAI	3	-112.68	34.50	4251 (1296)	--	--	--	--	9	0	0	NCDC
STANTON	YAVAPAI	3	-112.73	34.17	3480 (1061)	10	--	1967	12	25	2	2	NCDC
SYCAMORE RANGER STN	YAVAPAI	3	-111.97	34.35	4002 (1220)	8	--	1937	1	41	2	2	NCDC
TONTO SPRINGS RANGER STN 4 W	YAVAPAI	3	-112.75	34.62	4802 (1464)	24	--	1915	12	42	13	10	NCDC
TUZIGOOT	YAVAPAI	3	-112.35	34.77	3469 (1058)	--	--	--	--	29	0	0	NCDC
TUZIGOOT NATL MONUMENT	YAVAPAI	3	-112.03	34.75	3382 (1031)	26	--	1967	12	31	4	3	NCDC
WALNUT CREEK	YAVAPAI	3	-112.82	34.93	5089 (1551)	55	--	1948	2	91	14	11	NCDC
WALNUT GROVE	YAVAPAI	3	-112.57	34.32	3763 (1147)	14	--	1913	2	112	1	1	NCDC
YAEGER CANYON	YAVAPAI	3	-112.17	34.68	6002 (1830)	30	--	1933	1	32	31	25	NCDC
YAVA 6 ESE	YAVAPAI	3	-112.80	34.45	3782 (1153)	--	--	--	--	28	0	0	NCDC
BAKER BUTTE	GILA	4	-111.40	34.45	7300 (2226)	--	23.9	73	3	34	119	105	S. Course
Baker Butte	Gila	4	-111.40	34.45	7300 (2226)	--	14.7	83	3	23	90	81	Snotel
BAKER BUTTE NO. 2	GILA	4	-11.38	34.45	7700 (2348)	--	30.8	73	3	34	197	175	S. Course
BAR T BAR RANCH	GILA	4	-111.37	34.03	3103 (946)	--	--	--	--	25	0	0	NCDC
GISELA	GILA	4	-111.28	34.12	2899 (884)	19	--	1967	12	94	1	1	NCDC
GLOBE	GILA	4	-110.77	33.38	3649 (1113)	12	--	1985	2	25	2	1	NCDC
* GLOBE #2	GILA	4	-110.77	33.40	3749 (1143)	--	--	--	--	12	0	0	NCDC
GLOBE RANGER STN	GILA	4	-110.78	33.38	3552 (1083)	11	--	1967	12	76	1	1	NCDC
* GRAPEVINE	GILA	4	-111.05	33.63	2221 (677)	--	--	--	--	4	0	0	NCDC
* HILLTOP	GILA	4	-110.40	33.62	5704 (1739)	12	--	1945	3	6	15	13	NCDC
INTAKE	GILA	4	-110.93	33.62	2221 (677)	--	--	--	--	47	0	0	NCDC
MIAMI	GILA	4	-110.87	33.40	3559 (1085)	--	--	--	--	92	0	0	NCDC
* MILLERS RIM TRAIL RANCH	GILA	4	-111.23	34.40	5631 (1717)	--	--	--	--	1	0	0	NCDC
NATURAL BRIDGE	GILA	4	-111.45	34.32	4612 (1406)	36	--	1967	12	82	25	19	NCDC
* O W RANCH	GILA	4	-110.80	34.28	7203 (2196)	--	--	--	--	1	0	0	NCDC
* PARKER CREEK MNTC YRD	GILA	4	-110.95	33.80	5504 (1678)	--	--	--	--	1	0	0	NCDC
PAYSON	GILA	4	-111.33	34.23	4912 (1498)	48	--	1967	12	58	28	21	NCDC
PAYSON 12 NNE	GILA	4	-111.27	34.40	5504 (1678)	42	--	1967	12	25	35	30	NCDC
PAYSON RANGER STN	GILA	4	-111.33	34.23	4851 (1479)	24	--	1937	1	72	14	11	NCDC
* PAYSON RANGER STN 2	GILA	4	-111.30	34.25	5002 (1525)	--	--	--	--	3	0	0	NCDC
* PINE	GILA	4	-111.47	34.38	5452 (1662)	14	--	1974	1	2	23	20	NCDC
PLEASANT VALLEY R S	GILA	4	-110.95	34.10	5049 (1539)	27	--	1967	12	42	5	4	NCDC
PROMONTORY BUTTE	GILA	4	-111.02	34.37	7930 (2418)	--	35.4	80	4	17	216	194	S. Course
PUNKIN CENTER	GILA	4	-111.30	33.85	2325 (709)	--	--	--	--	33	0	0	NCDC
RENO R S	GILA	4	-111.32	33.87	2420 (738)	9	--	1967	12	59	0	0	NCDC
* RIM TRAIL RANCH	GILA	4	-111.27	34.40	5631 (1717)	--	--	--	--	2	0	0	NCDC
ROOSEVELT 1 WNW	GILA	4	-111.15	33.67	2204 (672)	--	--	--	--	101	0	0	NCDC
SALT RIVER	GILA	4	-110.50	33.80	3611 (1101)	12	--	1942	1	19	2	2	NCDC
SAN CARLOS	GILA	4	-110.45	33.35	2640 (805)	--	--	--	--	33	0	0	NCDC
* SAN CARLOS AIRPORT	GILA	4	-110.47	33.38	2889 (881)	--	--	--	--	4	0	0	NCDC
SAN CARLOS RESERVOIR	GILA	4	-110.53	33.18	2532 (772)	--	--	--	--	62	0	0	NCDC
* SENECA 3 NW	GILA	4	-110.53	33.78	4924 (1501)	--	--	--	--	2	0	0	NCDC
SIERRA ANCHA	GILA	4	-110.97	33.80	5099 (1555)	30	--	1967	12	50	20	16	NCDC
TONTO CREEK FISH HAT 2	GILA	4	-111.10	34.38	6388 (1948)	27	--	1997	1	31	15	12	NCDC
TONTO CREEK FISH HATCHERY	GILA	4	-111.10	34.37	6282 (1915)	58	--	1967	12	28	39	31	NCDC
WORKMAN CREEK	GILA	4	-110.92	33.82	6900 (2104)	--	20.1	73	3	42	102	90	S. Course
Workman Creek	Gila	4	-110.92	33.82	6900 (2104)	--	16.1	83	3	23	94	85	Snotel
* WORKMAN CREEK 1	GILA	4	-110.92	33.82	6973 (2126)	--	--	--	--	1	0	0	NCDC
* WORKMAN CREEK 2	GILA	4	-110.92	33.82	6973 (2126)	25	--	1942	1	2	55	43	NCDC
YOUNG	GILA	4	-110.93	34.10	5051 (1540)	31	--	1953	3	60	15	12	NCDC
* ALAMO 8 SW	LA PAZ	5	-113.70	34.20	951 (290)	--	--	--	--	4	0	0	NCDC
ALAMO DAM	LA PAZ	5	-113.58	34.23	1290 (393)	--	--	--	--	31	0	0	NCDC
* ALAMO DAM 6 ESE	YUMA	5	-113.47	34.25	1480 (451)	--	--	--	--	13	0	0	NCDC
* ALAMO RANGER STN	YUMA	5	-110.85	33.50	3040 (927)	--	--	--	--	4	0	0	NCDC
BOUSE	LA PAZ	5	-114.02	33.95	925 (282)	--	--	--	--	55	0	0	NCDC
DATELAND	YUMA	5	-113.53	32.80	449 (137)	--	--	--	--	17	0	0	NCDC
DATELAND WHITEWING RANCH	YUMA	5	-113.50	32.97	520 (159)	--	--	--	--	34	0	0	NCDC
EHRENBERG	YUMA	5	-114.53	33.60	322 (98)	--	--	--	--	30	0	0	NCDC
EHRENBERG 2 E	LA PAZ	5	-114.47	33.62	465 (142)	--	--	--	--	29	0	0	NCDC
* IMPERIAL DAM	YUMA	5	-114.47	32.88	171 (52)	--	--	--	--	2	0	0	NCDC
KOFA MINE	YUMA	5	-113.97	33.27	1774 (541)	--	--	--	--	54	0	0	NCDC
MOHAWK	LA PAZ	5	-113.77	32.73	541 (165)	--	--	--	--	52	0	0	NCDC
PARKER	LA PAZ	5	-114.28	34.15	420 (128)	--	--	--	--	113	0	0	NCDC
QUARTZSITE	LA PAZ	5	-114.23	33.67	875 (267)	--	--	--	--	37	0	0	NCDC
SALOME 1 ESE	YUMA	5	-113.62	33.78	1902 (580)	--	--	--	--	17	0	0	NCDC
* SALOME 17 SE	LA PAZ	5	-113.48	33.68	1599 (487)	--	--	--	--	12	0	0	NCDC
SALOME 6 SE	LA PAZ	5	-113.53	33.73	1703 (519)	--	--	--	--	50	0	0	NCDC

Station Name	County	Climate Division	Longitude	Latitude	Elev. (ft (m))	Max Snow Depth (in)	Max SWE (in of H <sub>2</sub> O)	Year of Max	Month of Max	# of Years of Data	50-yr Ground Snow Load (psf)	30-yr Ground Snow Load (psf)	Data Source
AGUILA	MARICOPA	6	-113.18	33.95	2164 (660)	--	--	--	--	78	0	0	NCDC
ALHAMBRA	MARICOPA	6	-112.12	33.52	1142 (348)	--	--	--	--	29	0	0	NCDC
APACHE JUNCTION	PINAL	6	-111.55	33.42	1722 (525)	--	--	--	--	15	0	0	NCDC
* APACHE JUNCTION 4 NNW	PINAL	6	-111.58	33.47	1890 (576)	--	--	--	--	5	0	0	NCDC
APACHE JUNCTION 5 NE	PINAL	6	-111.48	33.47	2069 (631)	--	--	--	--	19	0	0	NCDC
* ARIZONA CITY	PINAL	6	-111.67	32.75	1505 (459)	--	--	--	--	4	0	0	NCDC
* ARIZONA FALLS 1 WNW	MARICOPA	6	-111.98	33.50	1250 (381)	--	--	--	--	6	0	0	NCDC
ARIZONA FALLS 1 WNW	MARICOPA	6	-111.97	33.48	1250 (381)	--	--	--	--	16	0	0	NCDC
ASHURST HAYDEN DAM	PINAL	6	-111.28	33.08	1549 (472)	--	--	--	--	50	0	0	NCDC
BARTLETT DAM	MARICOPA	6	-111.65	33.82	1650 (503)	--	--	--	--	67	0	0	NCDC
BEARDSLEY	MARICOPA	6	-112.38	33.67	1270 (387)	--	--	--	--	29	0	0	NCDC
BUCKEYE	MARICOPA	6	-112.58	33.38	890 (271)	--	--	--	--	110	0	0	NCDC
CAREFREE	MARICOPA	6	-111.90	33.82	2529 (771)	--	--	--	--	38	0	0	NCDC
CASA GRANDE	PINAL	6	-111.72	32.88	1403 (428)	--	--	--	--	105	0	0	NCDC
CASA GRANDE NATL MONUMENT	PINAL	6	-111.53	33.00	1419 (433)	--	--	--	--	85	0	0	NCDC
* CAVE CREEK	PINAL	6	-111.95	33.83	2122 (647)	--	--	--	--	13	0	0	NCDC
* CAVE CREEK 3 ESE	MARICOPA	6	-111.90	33.82	2529 (771)	--	--	--	--	2	0	0	NCDC
CAVE CREEK DAM	MARICOPA	6	-112.05	33.72	1670 (509)	--	--	--	--	21	0	0	NCDC
CHANDLER	MARICOPA	6	-111.83	33.30	1220 (372)	--	--	--	--	33	0	0	NCDC
CHANDLER HEIGHTS	MARICOPA	6	-111.68	33.20	1425 (434)	--	--	--	--	58	0	0	NCDC
DEER VALLEY	MARICOPA	6	-112.08	33.58	1257 (383)	--	--	--	--	36	0	0	NCDC
* EAST MESA	MARICOPA	6	-111.65	33.42	1518 (463)	--	--	--	--	4	0	0	NCDC
* EL MIRAGE	PINAL	6	-112.32	33.63	1142 (348)	--	--	--	--	1	0	0	NCDC
ELOY 4 NE	PINAL	6	-111.52	32.78	1545 (471)	--	--	--	--	55	0	0	NCDC
FALCON FIELD	MARICOPA	6	-111.75	33.43	1322 (403)	--	--	--	--	29	0	0	NCDC
FLORENCE	PINAL	6	-111.38	33.03	1400 (427)	--	--	--	--	100	0	0	NCDC
FOUNTAIN HILLS	MARICOPA	6	-111.72	33.60	1580 (482)	--	--	--	--	27	0	0	NCDC
GILA BEND	MARICOPA	6	-112.72	32.95	735 (224)	--	--	--	--	111	0	0	NCDC
GILA BEND AIRPORT	PINAL	6	-112.72	32.88	853 (260)	--	--	--	--	23	0	0	NCDC
GOULDS RANCH	MARICOPA	6	-112.07	33.38	1201 (366)	--	--	--	--	46	0	0	NCDC
GRANITE REEF DAM	MARICOPA	6	-111.70	33.52	1322 (403)	--	--	--	--	173	0	0	NCDC
GRIGGS 3 W	MARICOPA	6	-112.48	33.50	1160 (354)	--	--	--	--	41	0	0	NCDC
HARQUAHALA PLAINS	PINAL	6	-113.17	33.53	1220 (372)	--	--	--	--	28	0	0	NCDC
HORSESHOE DAM	MARICOPA	6	-111.72	33.98	2019 (616)	--	--	--	--	58	0	0	NCDC
KEARNY	PINAL	6	-110.90	33.05	1830 (558)	--	--	--	--	22	0	0	NCDC
KELVIN	PINAL	6	-110.97	33.10	1850 (564)	--	--	--	--	37	0	0	NCDC
* LAKE PLEASANT	MARICOPA	6	-112.27	33.85	1601 (488)	--	--	--	--	10	0	0	NCDC
LAKE PLEASANT	MARICOPA	6	-112.27	33.83	1536 (468)	--	--	--	--	19	0	0	NCDC
LAVEEN 3 SSE	MARICOPA	6	-112.15	33.33	1135 (346)	--	--	--	--	58	0	0	NCDC
LITCHFIELD PARK	MARICOPA	6	-112.37	33.50	1030 (314)	--	--	--	--	86	0	0	NCDC
MARICOPA 4 N	PINAL	6	-112.03	33.12	1160 (354)	--	--	--	--	46	0	0	NCDC
MARICOPA 9 SSW	PINAL	6	-112.10	32.92	1401 (427)	--	--	--	--	58	0	0	NCDC
MARINETTE	MARICOPA	6	-112.30	33.63	1152 (351)	--	--	--	--	48	0	0	NCDC
MESA	MARICOPA	6	-111.82	33.42	1235 (376)	--	--	--	--	108	0	0	NCDC
* MONTEZUMA	MARICOPA	6	-113.38	33.10	741 (226)	--	--	--	--	5	0	0	NCDC
MORMON FLAT	MARICOPA	6	-111.45	33.55	1705 (520)	--	--	--	--	83	0	0	NCDC
MUMMY MOUNTAIN	MARICOPA	6	-111.97	33.55	1421 (433)	--	--	--	--	16	0	0	NCDC
ORACLE	PINAL	6	-110.78	32.60	4602 (1403)	16	--	1937	1	54	7	6	NCDC
ORACLE 2 SE	PINAL	6	-110.73	32.60	4509 (1375)	12	--	1967	12	56	2	2	NCDC
* PAINTED ROCK DAM	MARICOPA	6	-113.03	33.08	568 (173)	--	--	--	--	6	0	0	NCDC
* PARADISE VALLEY NO 2	MARICOPA	6	-111.97	33.57	1381 (421)	--	--	--	--	4	0	0	NCDC
* PARADISE VALLEY NO 2	MARICOPA	6	-111.95	33.55	1421 (433)	--	--	--	--	5	0	0	NCDC
PHOENIX CITY	MARICOPA	6	-112.08	33.45	1098 (335)	--	--	--	--	48	0	0	NCDC
PHOENIX INDIAN SCHOOL	MARICOPA	6	-112.07	33.50	1122 (342)	--	--	--	--	30	0	0	NCDC
PHOENIX SKY HARBOR INTL AP	MARICOPA	6	-112.00	33.43	1107 (337)	--	--	--	--	59	0	0	NCDC
* PHOENIX SOUTH MOUNTAIN	MARICOPA	6	-112.05	33.33	2645 (807)	--	--	--	--	9	0	0	NCDC
PICACHO 8 SE	PINAL	6	-111.40	32.65	1830 (558)	--	--	--	--	19	0	0	NCDC
PICACHO RESERVOIR	PINAL	6	-111.47	32.87	1512 (461)	--	--	--	--	28	0	0	NCDC
PINAL RANCH	PINAL	6	-110.98	33.35	4523 (1379)	25	--	1967	12	78	16	12	NCDC
* PINNACLE PEAK	MARICOPA	6	-111.87	33.73	2564 (782)	--	--	--	--	4	0	0	NCDC
RED ROCK 6 SSW	PINAL	6	-111.33	32.48	1879 (573)	--	--	--	--	53	0	0	NCDC
SACATON	PINAL	6	-111.75	33.08	1285 (392)	--	--	--	--	96	0	0	NCDC
SAN MANUEL	PINAL	6	-110.63	32.60	3459 (1055)	9	--	1958	11	52	1	1	NCDC
SCOTTSDALE	MARICOPA	6	-111.88	33.47	1201 (366)	--	--	--	--	18	0	0	NCDC
SENTINEL	PINAL	6	-113.22	32.87	689 (210)	--	--	--	--	31	0	0	NCDC
* SLATE MOUNTAIN	PINAL	6	-111.88	32.52	1932 (589)	--	--	--	--	4	0	0	NCDC
SOUTH PHOENIX	MARICOPA	6	-112.07	33.38	1155 (352)	--	--	--	--	45	0	0	NCDC
* STANFIELD	PINAL	6	-111.97	32.88	1312 (400)	--	--	--	--	2	0	0	NCDC
STEWART MOUNTAIN	MARICOPA	6	-111.53	33.55	1422 (433)	--	--	--	--	58	0	0	NCDC



Appendix A is an expanded table with a more complete presentation of the statistical data, including distribution parameters, for all of the sites. With the distribution parameters for each site the ground snow loads for any mean recurrence interval can be calculated by the methods presented in Section 3.3.

Although there is some inherent error in the calculation of ground snow load from depth, there are a few sites that give confidence to the methods used. For the Happy Jack location there are 3 sites; Snow Course, SnoTel and NCDC each have a representation. (Results for all 3 sites are shown only in Appendix A) The SnoTel site has only 6 years of records available and discounted as less reliable. The 50-year ground snow load value of the Snow Course (SWE) site and that of the NCDC (snow depth) site are very similar. The NCDC site has a 50-year ground snow load of 95 psf predicted from the snow depth with equation 3.12, while the Snow Course site has a 50-year ground snow load of 90 psf calculated directly from SWE. The prediction in this case is off by only 6%. McNary is another location with multiple sites that has an accurate prediction of the 50-year ground snow load based on snow depth. The Snow Course site has a value of 61 psf calculated directly from SWE data, while the NCDC site has a predicted value of 58 psf based on snow depth. This is an error of only 5%.

On the other hand, Fort Valley has a predicted value for its NCDC site of 92 psf and a calculated value for its Snow Course site of only 60 psf, a 53% error. This is evidence that equation 3.12 represents only the best-fit regression curve for the snow depth vs. ground snow load data, as shown in Figure 3.5. Although this curve fits the

relationship best, there is still error in the approximation. As stated in Section 3.4 it can be expected that the prediction will be conservative and unconservative for approximately half of the predictions.

### 3.6 OBSERVATIONS

In an attempt to find other relationships with the ground snow loads a plot was created for latitude vs. 50-year ground snow loads.

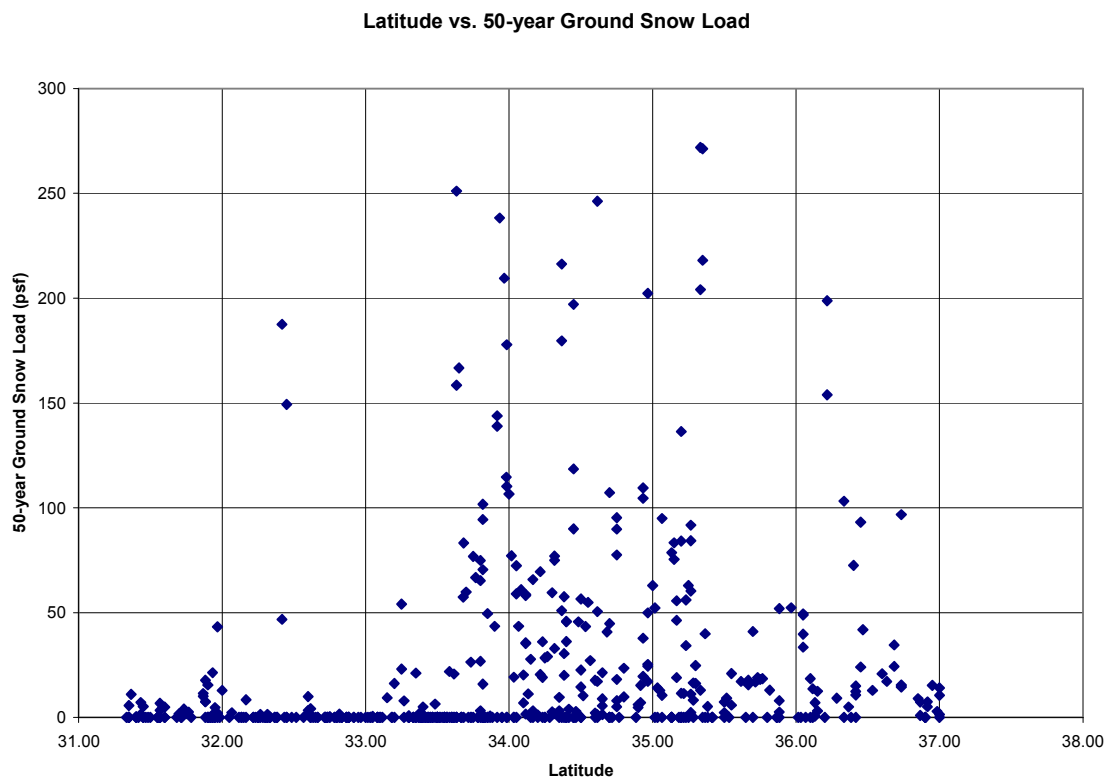


Figure 3.7  
Latitude vs. 50-year Ground Snow Load

As can be seen in Figure 3.7 there is no relationship evident between the latitude and ground snow load. Sites at all latitudes have a 50-year ground snow load of zero, and conversely, sites with 50-year ground snow loads are located at almost all latitudes.

Figures 3.8 through 3.13 illustrate the relationship between ground snow load and elevation separated by Climate Division. Climate Division 5 is not included since every site has a 50-year ground snow load of zero.

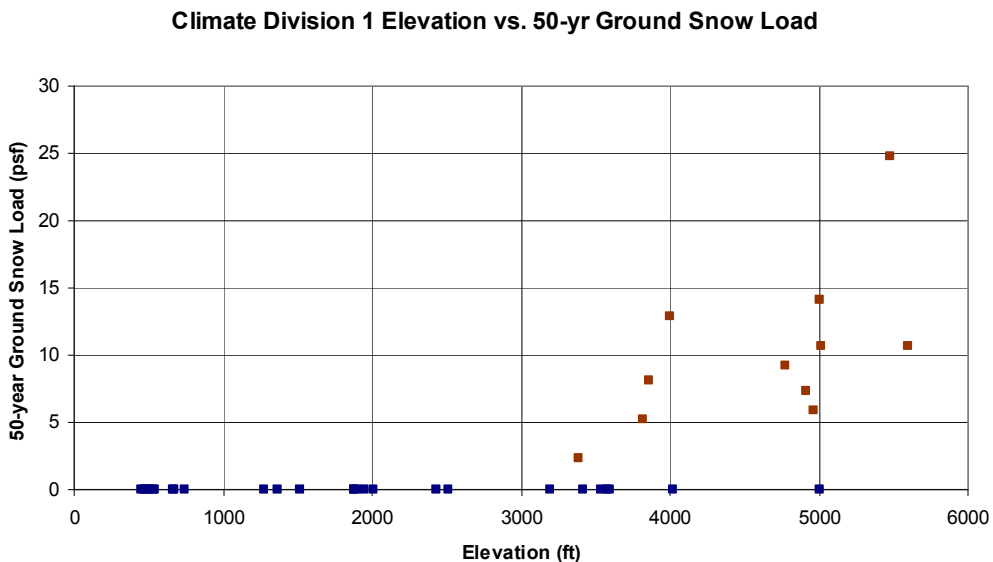


Figure 3.8  
Elevation vs. 50-year Ground Snow Load—Climate Division 1

**Climate Division 2 Elevation vs. 50-yr Ground Snow Load**

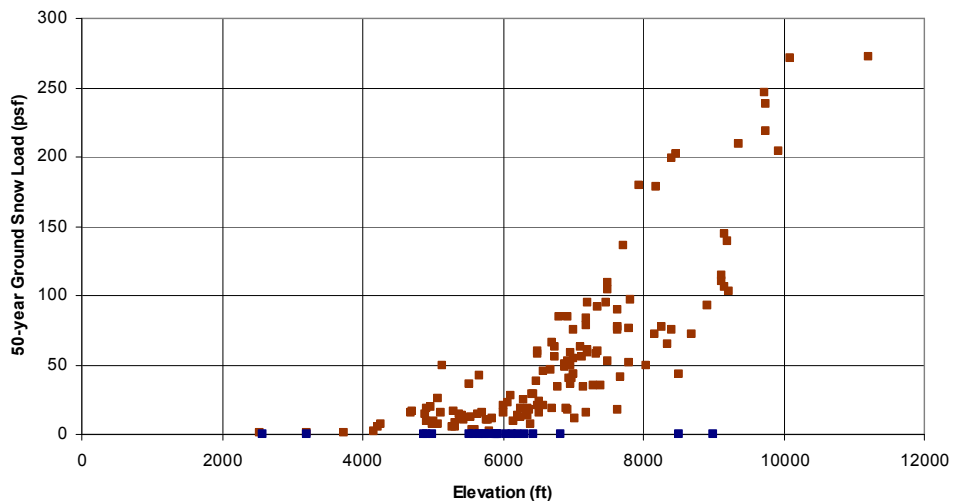


Figure 3.9  
Elevation vs. 50-year Ground Snow Load—Climate Division 2

**Climate Division 3 Elevation vs. 50-yr Ground Snow Load**

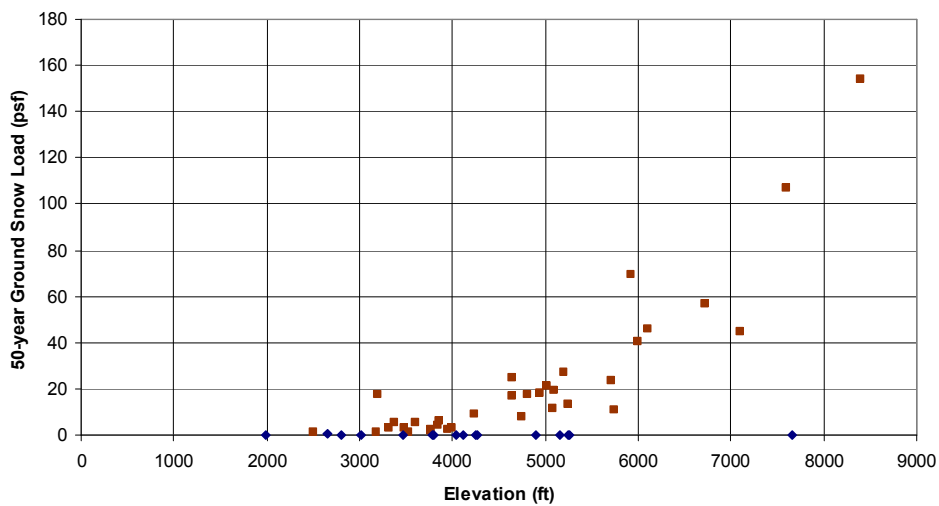
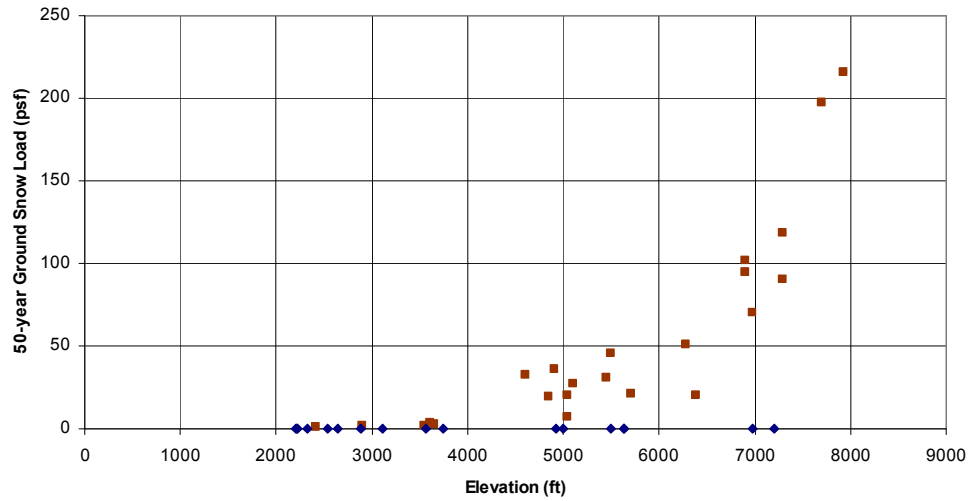


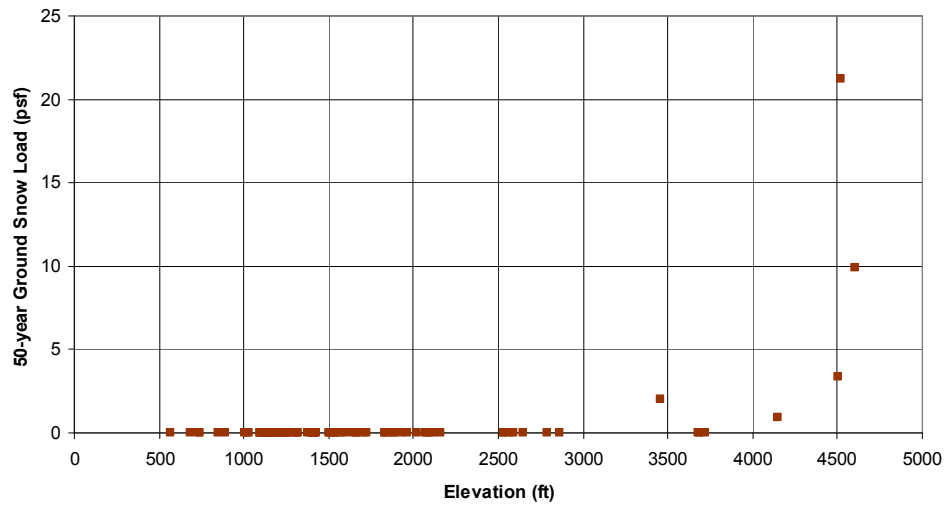
Figure 3.10  
Elevation vs. 50-year Ground Snow Load—Climate Division 3

**Climate Division 4 Elevation vs. 50-yr Ground Snow Load**



**Figure 3.11**  
Elevation vs. Ground Snow Load-Climate Division 4

**Climate Division 6 Elevation vs. 50-yr Ground Snow Load**



**Figure 3.12**  
Elevation vs. 50-year Ground Snow Load—Climate Division 6



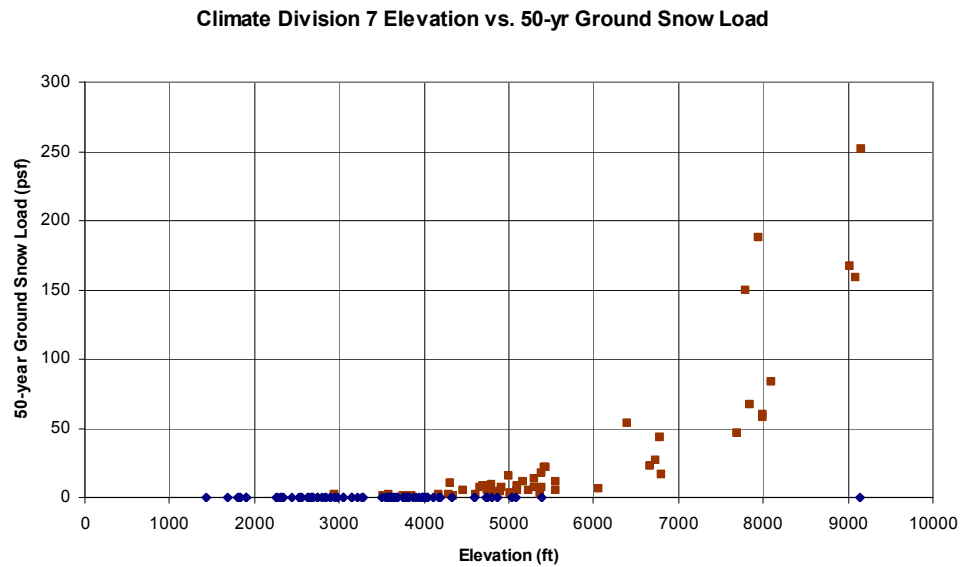


Figure 3.13  
Elevation vs. 50-year Ground Snow Load—Climate Division 7

As can be seen from the graphs in Figures 3.8 through 3.13, the obvious relationship exists that as elevation increases so does the 50-year ground snow load. There exist so much scatter and error that it is impossible to accurately quantify this relationship without significant error. In the future, when the Climate Divisions are reorganized to more closely align with homogeneous weather behavior these relationships may be more refined as well.

### 3.7 COMPARISON OF GROUND SNOW LOAD DATA

If some random sites are taken and ground snow load value comparisons are made between currently used resources and those values determined in this study the impact of this study can be determined. Table 3.5 shows this comparison.

Table 3.5  
Comparison of Ground Snow Loads

Site	Climate Division	Elevation (ft.)	Data Source for this study	Ground Snow Loads (psf)			
				A Review of Climatological Data for Ground Snow Loads in Arizona		ASCE 7-05 Ground Snow Load Map	Snow Load Data For Arizona
				50-year	30-year	50-year	30-year
Kingman AAF	1	3385	NCDC	2	2	5	20
Alpine	2	8048	NCDC	50	40	CS	50
Fredonia	2	4681	NCDC	15	13	CS	20
Happy Jack	2	7630	SC	90	79	CS	95
Pinetop 2E	2	7198	NCDC	59	51	CS	50
Gaddes Canyon	3	7600	SC	107	93	CS	100
Seligman	3	5249	NCDC	13	10	10	20
Payson 12NNE	4	5504	NCDC	46	39	CS	50
Oracle 2SE	6	4509	NCDC	3	3	5	20
Hannigan Meadows	7	9090	SC	159	141	CS	100
Sala Ranch	7	5163	NCDC	11	9	CS	20

In some instances the loads presented in this report are higher and in some cases lower than those listed in Snow Load Data for Arizona (Elliot, 1981). They are surprisingly close in most cases. For instance, Happy Jack had a recommended ground snow load of 95 psf in Snow Load Data for Arizona compared to a 50-year ground snow load from this study of 90 psf. In the case of Hannigan Meadows, however, the difference between the basic ground snow load in Snow Load Data for Arizona and this study is 59%. The ground snow load for Hannigan Meadows in this study is a result of 36 years of data with a COV of 0.56. It appears that this would give accurate results. This would lead to the conclusion that the previous data is unconservative. Out of the 10

sites considered only 3 had ground snow loads attainable from the ASCE 7-05 map. From this comparison it is evident that additional information could be very beneficial, and should be implemented among the structural design community for use in designing for snow in Arizona.

## **CHAPTER FOUR**

### **LIMITATIONS**

#### **4.1 MAPPING OF SNOW LOADS**

Many other states have developed a guide for ground snow loads specific to their state. Some of these states have mapped their ground snow loads with isolines, similar to what is presented in ASCE 7. (Idaho, 1984, Washington 1995) In the ASCE 7 ground snow load maps the isolines determine the depth directly. Another method developed to try to be even more accurate is to determine a relationship between the elevation and the ground snow load. Ground Snow Loads for Idaho contains a map where the contours or isolines separate coefficients to be used in an equation relating the elevation to the ground snow load (Sack and Sheik-Taheri, 1986). Not every location in between two isolines has the same ground snow load, but the elevation of the site and the contours together help to determine the ground snow load to be used for design. This is similar to what Elliott (1981) had in mind originally in Snow Load Data for Arizona as mentioned previously in Chapter 2.2. This would prove to be a very useful tool for designers if this relationship could be determined for each division of homogenous climate and advance software could be used to develop the isolines.

#### **4.2 MISSING DATA**

There is data presented in Snow Load Data for Arizona (Elliott, 1981) that could not be located. Some of the sites are not included in the NCDC database. There are most

likely other agencies that keep historical data on snow depth or snow water equivalents that were either not contacted or no response was received. Although efforts were made to be as comprehensive as possible, some locations were not represented in this study.

The recommendations in the ASCE 7-05 for conducting the Case Studies include finding the values for all known sites within a fifty-mile radius of the site of interest. For many locations along the borders of the state of Arizona this would require having the historical data for locations in Utah and New Mexico. For this report to be more complete it would make sense to include data available for all sites in New Mexico and Utah within fifty miles of the Arizona border. Nevada and California could be ignored since the elevations along the Arizona border adjacent to these two states is less than 4,000 feet above mean sea level and there are no records of significant snow fall for the Arizona sites in this area.

#### 4.3 STATISTICAL MODELING

When the data from the NCDC sites was manipulated and reduced for modeling the maximum snow depths were determined for a calendar year instead of a winter year. This may not have a consequential affect, except that if there was a large storm at the end of December the maximum snow depth for the calendar year of December and for the calendar year of January might be determined from the same storm. If the data was rearranged to be considered by winter years this theoretical end-of-the-year storm would be counted only once. The data from the NWCC was recorded by winter year, not

calendar year; hence this concern only applies to the data obtained from the NCDC database.

## *CHAPTER FIVE*

### SUMMARY AND CONCLUSION

#### 5.1 SUMMARY

Comprehensive structural design requires consideration for snow loads. Modern building codes determine snow loads on structures by applying coefficients for exposure, roof shape, etc. to basic ground snow loads. Basic ground snow loads are currently loads with a mean recurrence interval of 50 years and are illustrated in ground snow load maps found in the code. These maps contain areas with a “CS” designation indicating that a site-specific Case Study is required to determine the basic ground snow load within that area. A snow load design guide was published for Arizona in 1973 with a second printing in 1981. Nothing has been done since this time to update the snow loads presented in that guide. Although it is a great guide for designers, it is not completely consistent with current code and modern standards of practice, e.g., the loads published are presented as having a 30-year mean recurrence interval.

Research over the past 25 years has indicated that modeling annual extreme snow depth and SWE with a lognormal distribution is a reliable approach. Advances have also been made in finding an appropriate snow depth-density relationship. To update available ground snow loads historical Snow Water Equivalent data and snow depth data were collected for over 500 sites in Arizona. The annual maximum SWE or snow depth was gleaned from the data and the records for each site were modeled as a lognormal distribution to determine the SWE or snow depth with a 50-year mean recurrence interval. For the sites with SWE data, the 50-year ground snow load was calculated using

the weight of water multiplied by the SWE. A snow density prediction relating snow density to depth was determined from historical records for 41 sites that contained both SWE and snow depth data. For the sites with snow depth data, this relationship was applied to the 50-year snow depth to determine the 50-year ground snow load. Table 3.4 is a summary of sites across Arizona with their respective 50-year ground snow loads. For convenience and comparison, the 30-year ground snow loads are listed also. Further work could be done to improve this report as a design guide by developing a contour map for the 50-year basic ground snow loads in Arizona.

## 5.2 CONCLUSION

This report presents improved basic ground snow loads for Arizona consistent with recent research and modern building code standards. The attempt of this study was to provide up-to-date ground snow loads to be used in structural design in Arizona and fill some of the voids in current design practices with a scientific approach of applying reliable statistical methods to historical climatological data. Inasmuch as the historical data is accurate and the statistical methods reliable the ground snow loads presented herein are satisfactory for use in structural design and should be encouraged in the structural design community in Arizona.

These loads are believed to be accurate from a statistical modeling stand point, but caution should be used when applying the information presented in this report to structural design. In the immortal words of Elliott (1981) “Statistics are helpful, Judgment is essential.”



*APPENDIX A*





Ground Snow Loads with Statistical Data for All Sites

Table with columns: Station Name, County, Climate Division, Longitude, Latitude, Elev. (ft (m)), Max Snow Depth (in), Max SWE (in of H2O), Year of Max, Month of Max, # of Years of Data, 50-yr Ground Snow Load (psf) [Eqn. 2.1], 50-yr Ground Snow Load (psf) [Eqn. 3.12], 30-yr Ground Snow Load (psf) [Eqn. 2.1], 30-yr Ground Snow Load (psf) [Eqn. 3.12]. Rows include various sites like FREDONIA, GANADO, GRAND CANYON, etc.

\* indicates less than 15 years of data available

SHADED indicates 50-year Snow Load less than 12 PSF (negligible in typical design)





















Ground Snow Loads with Statistical Data for All Sites

Station Name	County	Climate Division	Longitude	Latitude	Elev. (ft (m))	Max Snow Depth (in)	Max SWE (in of H <sub>2</sub> O)	Year of Max	Month of Max	# of Years of Data	50-yr Ground Snow Load (psf) [Eqn. 2.1]	50-yr Ground Snow Load (psf) [Eqn. 3.12]	30-yr Ground Snow Load (psf) [Eqn. 2.1]	30-yr Ground Snow Load (psf) [Eqn. 3.12]
SABINO CANYON	PIMA	7	-110.82	32.32	2639 (805)	--	--	--	--	35	0	0	0	0
SAFFORD	GRAHAM	7	-109.72	32.83	2903 (885)	--	--	--	--	50	0	0	0	0
SAFFORD AGRI CENTER	GRAHAM	7	-109.68	32.82	2953 (900)	14	--	1967	12	57	1	2	1	1
SAHUARITA 2 NW	PIMA	7	-110.97	31.97	2689 (820)	--	--	--	--	17	0	0	0	0
* SAHUARITA 8 W	PIMA	7	-111.07	31.90	3559 (1085)	--	--	--	--	6	0	0	0	0
SALA RANCH	COCHISE	7	-109.98	31.87	5163 (1574)	11	--	1967	12	32	8	11	6	9
SAN RAFAEL RANCH	SANTA CRUZ	7	-110.62	31.35	4743 (1446)	--	--	--	--	53	0	0	0	0
SAN SIMON	COCHISE	7	-109.23	32.27	3609 (1100)	--	--	--	--	74	0	0	0	0
* SAN SIMON 5 NW	7	-109.27	32.33	3611 (1101)	--	--	--	--	--	13	0	0	0	0
SAN SIMON 9 ESE	COCHISE	7	-109.08	32.17	3879 (1183)	--	--	--	--	25	0	0	0	0
* SAN SIMON 9 NE	7	-109.13	32.37	4002 (1220)	--	--	--	--	--	2	0	0	0	0
SANTA MARGARITA	7	-111.58	31.68	3933 (1199)	--	--	--	--	--	34	0	0	0	0
SANTA RITA EXP RANGE	PIMA	7	-110.85	31.77	4299 (1311)	8	--	1951	1	56	2	3	1	2
SANTA ROSA SCHOOL	PIMA	7	-112.05	32.32	1840 (561)	--	--	--	--	19	0	0	0	0
SASABE	PIMA	7	-111.55	31.48	3589 (1094)	--	--	--	--	47	0	0	0	0
SASABE 6 NNE	PIMA	7	-111.50	31.57	3494 (1065)	--	--	--	--	18	0	0	0	0
SASABE 7 NW	PIMA	7	-111.60	31.60	3824 (1166)	--	--	--	--	55	0	0	0	0
SELLS	PIMA	7	-111.88	31.92	2345 (715)	--	--	--	--	35	0	0	0	0
SIERRA VISTA	COCHISE	7	-110.28	31.55	4599 (1402)	--	--	--	--	24	0	0	0	0
SILVER BELL	7	-111.50	32.38	2739 (835)	--	--	--	--	--	37	0	0	0	0
STEPHENS RANCH	COCHISE	7	-109.20	31.40	3998 (1219)	10	--	1980	2	55	0	0	0	0
* TANQUE R9 ON W4	7	-109.62	32.62	3562 (1086)	--	--	--	--	--	1	0	0	0	0
TOMBSTONE	COCHISE	7	-110.05	31.70	4609 (1405)	10	--	1997	1	109	1	2	1	1
TUCSON 17 NW	PIMA	7	-111.20	32.25	2560 (781)	--	--	--	--	24	0	0	0	0
TUCSON CAMP AVE EXP FM	PIMA	7	-110.95	32.28	2329 (710)	--	--	--	--	56	0	0	0	0
TUCSON INTERNATIONAL AP	PIMA	7	-110.95	32.13	2548 (777)	--	--	--	--	58	0	0	0	0
TUCSON MAGNETIC OBSY	PIMA	7	-110.83	32.25	2525 (770)	--	--	--	--	48	0	0	0	0
* TUCSON MOUNTAIN PARK	PIMA	7	-111.17	32.25	2850 (869)	--	--	--	--	7	0	0	0	0
* TUCSON MOUNTAIN PARK	7	-111.13	32.22	2679 (817)	--	--	--	--	--	1	0	0	0	0
* TUCSON NURSERY 4 NW	7	-111.05	32.30	2250 (686)	--	--	--	--	--	1	0	0	0	0
TUCSON WFO	PIMA	7	-110.95	32.23	2434 (742)	--	--	--	--	112	0	0	0	0
TUMACACORI NATL MONMNT	SANTA CRUZ	7	-111.05	31.57	3266 (996)	--	--	--	--	58	0	0	0	0
TUSCON U OF A #1	PIMA	7	-111.00	32.25	2314 (706)	--	--	--	--	24	0	0	0	0
* VAIL 7 N	PIMA	7	-110.72	32.13	2979 (908)	--	--	--	--	13	0	0	0	0
* WAHAK HOTRONTK	PIMA	7	-112.37	32.22	1902 (580)	--	--	--	--	4	0	0	0	0
* WHITLOCK VALLEY R2 ON W1	7	-109.52	32.82	3290 (1003)	--	--	--	--	--	1	0	0	0	0
Wildcat	Greenlee	7	-109.48	33.77	7850 (2393)	--	11.8	93	3	21	67	67	59	59
WILLCOX	COCHISE	7	-109.85	32.27	4174 (1273)	9	--	1912	2	107	1	1	1	1
Y LIGHTNING RANCH	COCHISE	7	-110.23	31.45	4589 (1399)	--	--	--	--	67	0	0	0	0

\* indicates less than 15 years of data available  
 SHADED indicates 50-year Snow Load less than 12 PSF (negligible in typical design)

Ground Snow Loads with Statistical Data for All Sites

Station Name	Data Source	Coop ID	# Years Zero Data	Min Snow Depth (in)	Min SWE (in of H <sub>2</sub> O)	Mean	Standard Dev.	Median	Mode	Skewness	COV	COV (truncated)	$\lambda_x$	$\zeta_x$	50-yr Snow Depth (in)	50-yr SWE (in of water)	30-yr Snow Depth (in)	30-yr SWE (in of water)
SABINO CANYON	NCDC	27355	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SAFFORD	NCDC	27388	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SAFFORD AGRI CENTER	NCDC	27390	44	0	--	0.7	2.0	0.0	0.0	5.34	3.07	1.00	-0.752	0.833	2.6	--	2.2	--
SAHUARITA 2 NW	NCDC	27403	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* SAHUARITA 8 W	NCDC	27419	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SALA RANCH	NCDC	27445	8	0	--	3.2	3.1	2.0	0.0	0.82	0.96	0.96	0.834	0.807	12.1	--	10.1	--
SAN RAFAEL RANCH	NCDC	27555	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SAN SIMON	NCDC	27560	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* SAN SIMON 5 NW	NCDC	27563	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SAN SIMON 9 ESE	NCDC	27567	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* SAN SIMON 9 NE	NCDC	27565	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SANTA MARGARITA	NCDC	27583	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SANTA RITA EXP RANGE	NCDC	27593	40	0	--	1.0	2.0	0.0	0.0	2.35	2.05	1.00	-0.365	0.833	3.8	--	3.2	--
SANTA ROSA SCHOOL	NCDC	27600	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SASABE	NCDC	27619	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SASABE 6 NNE	NCDC	27625	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SASABE 7 NW	NCDC	27622	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SELLS	NCDC	27726	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SIERRA VISTA	NCDC	27880	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
SILVER BELL	NCDC	27915	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
STEPHENS RANCH	NCDC	28205	53	0	--	0.2	1.4	0.0	0.0	7.03	6.28	1.00	-1.869	0.833	0.9	--	0.7	--
* TANQUE R9 ON W4	NCDC	28409	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
TOMBSTONE	NCDC	28619	90	0	--	0.7	1.9	0.0	0.0	2.86	2.58	1.00	-0.656	0.833	2.9	--	2.4	--
TUCSON 17 NW	NCDC	28795	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
TUCSON CAMP AVE EXP FM	NCDC	28796	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
TUCSON INTERNATIONAL AP	NCDC	28820	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
TUCSON MAGNETIC OBSY	NCDC	28800	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* TUCSON MOUNTAIN PARK	NCDC	28805	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* TUCSON MOUNTAIN PARK	NCDC	28806	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* TUCSON NURSERY 4 NW	NCDC	28810	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
TUCSON WFO	NCDC	28815	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
TUMACACORI NATL MONMNT	NCDC	28865	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
TUCSON U OF A #1	NCDC	28817	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* VAIL 7 N	NCDC	28998	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* WAHAK HOTRONTK	NCDC	29109	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
* WHITLOCK VALLEY R2 ON W1	NCDC	29279	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--
Wildcat	ST	866	0	--	1	4.9	2.9	4.7	6.3	0.69	0.59	0.59	1.434	0.544	--	12.8	--	11.4
WILLCOX	NCDC	29334	82	0	--	0.6	1.4	0.0	0.0	3.25	2.34	1.00	-0.845	0.833	2.4	--	2.0	--
Y LIGHTNING RANCH	NCDC	29562	--	0	--	0	--	--	--	--	--	--	--	--	0	--	0	--

\* indicates less than 15 years of data available  
 SHADED indicates 50-year Snow Load less than 12 PSF (negligible in typical design)

***APPENDIX B***



# SNOW LOAD DATA for ARIZONA



STRUCTURAL ENGINEERS ASSOCIATION  
OF ARIZONA

SNOW LOAD DATA FOR ARIZONA

Published By

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Mac Elliott

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FORWARD TO SECOND PRINTING OF SNOW LOAD DATA FOR ARIZONA. NOVEMBER, 1981

This Report originally advised caution in the use of the tabulated 30 year snow load figures. Caution is still advised. However, nothing has come to the attention of the Committee during the 8 years since the first printing to warrant significant revisions of the tabulated loads. Therefore, tables, charts, and text are left unchanged for this second printing.

It is strongly recommended that snow load records continue to be monitored, and compared with the data in this book. Individual changes should be made where warranted. Also, data from new stations is becoming available. Sometime in the future a thorough review of all the then available information, with subsequent updating and/or expansion of these tables may be advisable.

Perhaps the expanded data base available in the future would permit more formalized statistical projections, along the lines of the Weibull and Log Pearson Type III distributions used in much of the recent snow load work in other areas. However, even with such statistical approaches, considerable judgement will still be necessary in arriving at projected loads for specific sites. Arizona's winter weather is too variable. -- e.g. Weather coming from the Pacific Coast over a highly variable terrain and subject to a high degree of modification on the way, -- A warm State with a rain vs. snow picture extremely spotty and variable from year to year, -- Local conditions varying widely from point to point, -- etc.. Statistics are helpful, but judgement is essential.

The question has been raised as to how the tabulated 30 year loads relate to probable 50 and 100 year loads. In answer, most distributions used for hydrological predictions would show the 50 year recurrent load as about 15% higher than the 30 year load, and the 100 year recurrent load as about 25% greater than the 30 year load. These figures also seem reasonable for snow.

One more question that keeps coming up is how to turn inches of snow into weights on the ground. As stated in the Report, rules of thumb can be misleading. Densities routinely vary between 5% and 50% water. However, certain numbers may be useful as a rough guide in the higher, colder areas, where substantial snow pack remains for much of the winter. December, 10% to 20% water, January, 15% to 25% water, February, 20% to 30% water, March, 30% to 40% water, and April, 35% to 45% water.

Mac Elliott

## SNOW LOAD DATA FOR ARIZONA

### INTRODUCTION:

For many years a need has existed for a guide to aid in estimation of the weight of snow which might be expected to occur on structures in the Arizona high country. This report is intended to serve as such a guide.

### SCOPE:

The report is based on available records of snow depths and water contents. Basic ground and roof snow loads representing probable 30 year maximums are developed, and a detailed listing of these loads is given for various reporting stations around the State.

Also included are recommendations for roof design load modifications due to the following factors: wind removal of roof snow, roof slopes, unbalanced loads, roof valleys, multi-level roofs, roof projections, and ice loads.

### SOURCES OF DATA:

Two basic sets of records are available from which maximum snow loads may be estimated. U. S. Soil Conservation Service Snow Surveys and U. S. Weather Bureau records.

Soil Conservation Service (S.C.S.) readings are made twice a month and date back to 1938 at some stations. Measurements are taken for the most part away from populated areas, which somewhat limits their direct applicability to building in these areas. However, the data is extremely valuable due to inclusion of actual measurements of water content in the snow. Depths of snow are also recorded.

Available Weather Bureau (W.B.) records on the other hand list only snow depths, not water content, but do include reporting stations for nearly all of Arizona's inhabited communities. Some records date back to 1895, but longevity as well as completeness of these records varies greatly throughout the State.

Map No. 2 shows the Weather Bureau reporting stations and Map No. 3 the S.C.S. snow courses.

### SOURCES OF DATA (Continued)

A third source of information is the report, "Actual Snow Loads in Arizona" by H. M. Elliott. This is a detailed study of the great storm of December 13 - 20, 1967, and lists snow loads in pounds per square foot as well as the snow depths for 111 different reporting stations around the State. As this storm produced the heaviest short period snowfall on record in most areas of the State, the report provides a useful guide in arriving at basic ground load criteria.

See Bibliography at end of this report for additional sources of information.

### CONVERSION OF SNOW DEPTHS TO LOADS:

The foregoing data sources were searched to obtain the maximum recorded snow depths or loads at all reporting areas. (See Tables 1 - 5.) The snow depths without loads from the Weather Bureau records, and from a few incomplete Soil Conservation records, then had to be converted into pounds per square foot on the ground.

Any conversion at this time from depths listed on a printed page to maximum weights that existed on the ground years ago is fraught with error. To illustrate the problem, two listings are included for Hawley Lake. They give both the record maximum depth of 91" with a measured weight of 57 p.s.f. for 1967 (12% water), and the record maximum weight of 103 p.s.f. with a depth of only 45" for 1973, (44% water). These weights were not estimates, they were actual S.C.S. measurements. It is apparent that in spite of all the theoretical conversion data available in the literature (from snow depths to p.s.f.), an educated guess is the best we can hope for.

The process used to convert listed snow depths into pounds per square foot at stations without recorded loads was to search S.C.S. and Elliott data for comparable conditions. (S.C.S. and Elliott data contain both snow depths and p.s.f. loads.) Comparing this information with the depth at the station in question, a p.s.f. snow load estimate was made for that station. All listed Weather Bureau maximum p.s.f. data was arrived at in this manner, plus that for a few S.C.S. stations noted with asterisks (\*). There is no way of judging accuracies of these weight estimates, but hopefully they are within 30%±

## DEVELOPMENT OF BASIC GROUND SNOW LOADS:

Considerable effort was expended in an attempt to group geographical areas into "Snow Zones" so that meaningful snow load-to-elevation relationships could be developed for each zone. For convenience, areas used by the U.S. Weather Bureau in their Statewide reporting service were utilized in this grouping, since each such area has its own weather pattern. See Map No. 1.

Maximum recorded or estimated snow loads were plotted against elevation above sea level for each grouping. See Figs. 1 - 5. Curves were then drawn, generally but not always, through the high side of the plotted points. The final division of the State into five snow load zones provided helpful load-to-elevation curves for Zones I and V, and to a lesser degree for Zones II, III and IV.

The original intent was to use these curve values as Basic Ground Loads, similar to the approach taken in the Oregon and Colorado reports, (See Bibliography Nos. 4 and 5). However, due to the wide scatter of data this approach was finally abandoned. Each site was considered individually and a Basic Ground Load assigned accordingly. See Tables 1 - 5. The curves were used only as aids in arriving at Basic Ground Loads, and no curves have been reproduced with this report for fear of misleading.

This wide data scatter suggests the importance of considering all the pertinent features of an individual site rather than just the elevation. For example, south slopes and exposure to sun are very effective in reducing long term snow buildup. Flagstaff, with its south exposure, had almost no long term buildup during the spring of 1973, while Newman Park, 15 miles south hit 75 p.s.f. and Happy Jack, 35 miles south reached 105 p.s.f. on the ground.

A study of the Statewide storm pattern shows the higher elevations of Zones II, III and IV, "stripping" most of the snow out of winter storms before it reaches Zone I. Snow loads are light in the northeast, "Four Corners" area. (There was no reporting Zone I station above 7,500 feet, but a snow load increase seems possible above this elevation. An estimate of 30 p.s.f. Basic Ground Snow Load at 8,000 feet seems reasonable for Zone I.)

Due to scarcity of data and wide variation of conditions, structures above 8,500 ft. in Zones I and V, and above 10,000 ft. in Zones II, III and IV should have special investigations to determine design snow loads.

DEVELOPMENT OF BASIC GROUND SNOW LOADS (Continued)

Normal roof live loads should govern over snow loads below about 4,500 feet at Zones I and V, and below about 3,000 feet at Zones II, III and IV.

MISCELLANEOUS FACTORS AFFECTING RELIABILITY:

Probably the two factors most adversely affecting reliability have been shortage of data and the necessity of estimating snow weights from Weather Bureau depth records. Unfortunately for the purposes of this report, the populated areas containing most of the building activity do not have records of actual snow weight measurements. To compensate for these shortages, as much nearby data, (S.C.S., W.B., or H.M.E.) was considered as seemed applicable in arriving at Basic Loads for each specific station.

One specific factor tending towards the unconservative was as follows: Elliott utilized the daily water precipitation records of the Weather Bureau in his analysis of the December 1967 storm. Subsequent conversations with the Weather Bureau indicate that during heavy snow storms the amount of measured precipitation may be less than the actual precipitation, perhaps by 10 to 30%, due to losses in collecting and melting the snow. It is therefore quite possible that some of Elliott's loads were low.

However, on the conservative side, it seems much of the "hard" data, (obtained from Soil Conservation measurements and Elliott's report, and consisting of actual snow weights, not just depth measurements), represented something greater than a 30 year maximum.

The effects of the above factors were all estimated when arriving at Basic 30 Year Loads. No effort was made to err on either the conservative or unconservative side.

See also Conclusions at end of this report.



REDUCTIONS FOR WIND REMOVAL OF ROOF SNOW:

Wind can blow snow off roofs, and many codes make allowances for this. These allowances vary considerably however, between different geographical areas.

- a. The Canadian code allows for snow blown off roofs by using a basic coefficient of 0.80. (Their roof load is assumed equal to 80% of the ground snow load.) Also allowed is an ultimate reduction down to 60% of ground snow if the roof is totally exposed to the wind on all sides.
- b. Oregon allows approximately the same reductions as Canada, except in areas west of the Cascades where due to wetter snow and gentler winds further reduction to 60% is not allowed.
- c. The California Division of Architecture allows no reduction for wind removal of roof snow. At one time they did allow a reduction down to 80% of the ground snow load, but extensive recent measurements seemed not to justify the reduction.

Unfortunately, there are no known available records of comparisons between roof and adjacent ground snow loads in Arizona, so experience in other areas must be utilized. In attempting to arrive at a recommendation for Arizona, the following items seemed pertinent:

- a. Canadian winters are longer and colder than Arizona winters, giving more time for snow to be blown off Canadian roofs as well as colder, drier easier snow to blow off.
- b. Canadian winter winds are generally stronger than Arizona winter winds.
- c. Many of Arizona's maximum snow loads were recorded during the December 1967 storm of Elliott's report. This was a short period storm with little opportunity for blow off.
- d. At many locations in Arizona (the lower elevations), maximum loads consist of wet sticky snow delivered during short period storms, with little chance for blow off. (Temperatures at these elevations tend to warm up and prevent long term build up between storms.)

REDUCTIONS FOR WIND REMOVAL OF ROOF SNOW (Continued)

The foregoing four points all argue against allowing as much wind reduction for Arizona as for Canada. However, not all Arizona snow is wet and sticky, and it would seem reasonable to allow reductions at higher altitudes in the colder parts of the State. Reductions equal to the Canadian wind reductions would seem reasonable at elevations above about 7,500 feet in the northern part of the State, (Zones I, II, III and IV). Reductions equal to say one half of the Canadian wind reductions would seem reasonable at elevations from about 6,000 feet to 7,500 feet in the north (Zones I, II, III and IV), and above about 7,000 feet in the south (Zone V).

In Zones I, II, III and IV, this would result in a Basic Roof Load equal to 80% of the Ground Snow Load at elevations above 7,500 feet, and equal to 90% of the Ground Snow Load between 6,000 feet and 7,500 feet. At Zone V Basic Roof Loads would be 90% of Ground Snow Loads at elevations above 7,000 feet. These values are listed in Tables 1 - 5. Also allowed would be further reductions for roofs fully exposed to wind on all sides, as explained in the Table footnotes.

None of the above reductions should be applied at lower elevations.

MODIFICATIONS DUE TO ROOF SLOPES, UNBALANCED LOADS, ROOF VALLEYS, MULTI LEVEL ROOFS, ROOF PROJECTIONS AND ICE LOADS:

The Canadian Building Code has recommendations for dealing with the above factors. Since Arizona conditions differ somewhat, as previously discussed under "Wind Removal of Roof Snow", the Canadian recommendations had to be modified to fit Arizona. Figs. C2-1 through C2-7 are patterned after the Canadian format as closely as possible. Coefficients are given to determine load patterns on the roof.

Caution!! As explained in Figs. C2-1 through C2-7, there are two types of coefficients, those with asterisks ( $C_s^*$ ), and those without asterisks (C). Coefficients  $C_s^*$  are to be multiplied by the Basic Roof Snow Loads and coefficients C are to be multiplied by the Basic Ground Snow Loads in order to arrive at the adjusted roof loadings. (Basic Roof Loads given in Tables 1 - 5, including the further reductions for exposed buildings at higher elevations per footnotes in the Tables, are applicable to  $C_s^*$ .)

- a. Sloped Roofs: See Case I of Figs. C2-1, C2-2 and C2-3 for allowable reductions.

MODIFICATIONS DUE TO ROOF SLOPES, UNBALANCED LOADS, ROOF VALLEYS,  
MULTI LEVEL ROOFS, ROOF PROJECTIONS AND ICE LOADS (Continued)

- b. Unbalanced Loads: Peaked and curved roofs produce an aerodynamic shade on the lee side. Snow from the windward side is blown over and dropped on the lee slope, building up an unbalanced load. See Case II at Figs. C2-2 and C2-3 for load distributions.
- c. Roof Valleys: See Fig. C2-4 for load concentrations at roof valleys.
- d. Multi Level Roofs: Lower roofs may build up snow which has either drifted or slid down from adjacent higher roofs. See Figs. C2-5 and C2-6 for load distributions.
- e. Roof Projections: Snow may build up adjacent to roof projections. See Fig. C2-7 for distribution.
- f. Ice Loads at Roof Edges: In addition to snow loads, ice loads should be applied at edges of sloped roofs. The following amounts appear reasonable for average conditions, but special conditions should receive special evaluation.

<u>Basic Ground Snow Load (p.s.f.)</u>	<u>Ice Load Per Lin Ft at Lower Edge of Sloped Roof</u>
20 - 30	50 p.l.f.
30 - 50	75 p.l.f.
Above 50	100 p.l.f.

GENERAL:

1. Skip Loading. All roof areas should have design snow load applied
  - a. with full load on entire area or
  - b. with full load on any portion of the area and zero load on the remainder,

whichever produces maximum stress on the member concerned. This is to guard against effects of partial snow removal, as well as recognizing the fact that snow loads are often uneven. This requirement applies to conditions shown in Figs. C2-1 through C2-7, as well as to all other snow loads.

GENERAL (Continued)

2. A distinction should be noted between say a normal 20 p.s.f. live load and a 20 p.s.f. snow load. The live load may be reduced according to roof slopes and tributary areas per building code allowances, whereas the 20 p.s.f. snow load is not subject to the same reductions.
3. Beware of rules of thumb for converting maximum snow depth into snow loads. New fallen snow at high cold areas can easily have only 5% water, while older snow may run up to 50% water. Water percentages are influenced by such factors as temperature of formation, temperature record on the ground, subsequent snowfalls, subsequent rain, depth of snow available to catch and refreeze melt or subsequent rain, clouds, sun, shade, weight pressing on the lower layers, etc.
4. Actual snow weight data (not just depths) will continue to be difficult to obtain at many areas. Observers are for the most part unpaid volunteers who have many other concerns, particularly during times of heavy storms, than the measurement of the water content of the snow on the roof.
5. Snow removal during heavy storms is unreliable. Streets impassable. Manpower blocked inside homes, etc. Designers cannot count on snow removal.

CONCLUSIONS:

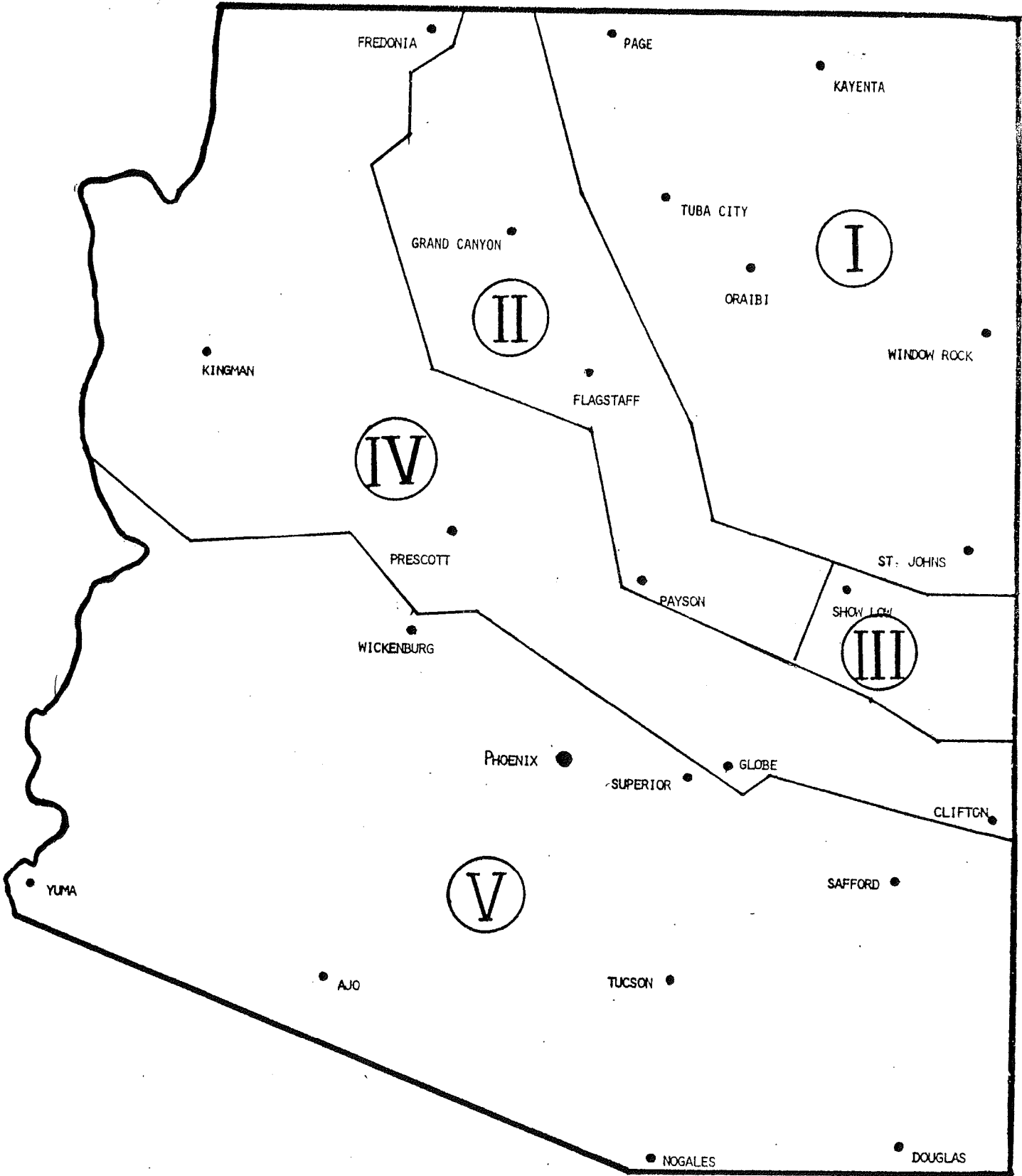
Until some future, more refined study is made, the Basic Snow Loads developed in this report seem to be a reasonable estimate of the maximum snow loads that might be expected over a 30 year period. As such, they could serve as a guide for design loads for structures. However, while as much information as possible was searched and reasonable care was used in the preparation of this report, there is obviously no way to guarantee that the loads listed will not be exceeded in the next 30 years. As stated earlier, no attempt was made to err on the conservative side. And no claim is made to Divine Revelation.

The listed values should be treated only as a guide. The designer must use his best judgment. Attention must be paid to local conditions that might cause increases; e.g. north slopes, shade, drifting, wind shelter that would prevent snow from blowing off roofs, etc., and particularly to any known history of heavier snow. And snow loads for structures requiring a high level of safety should always receive special consideration.

PREPARATION:

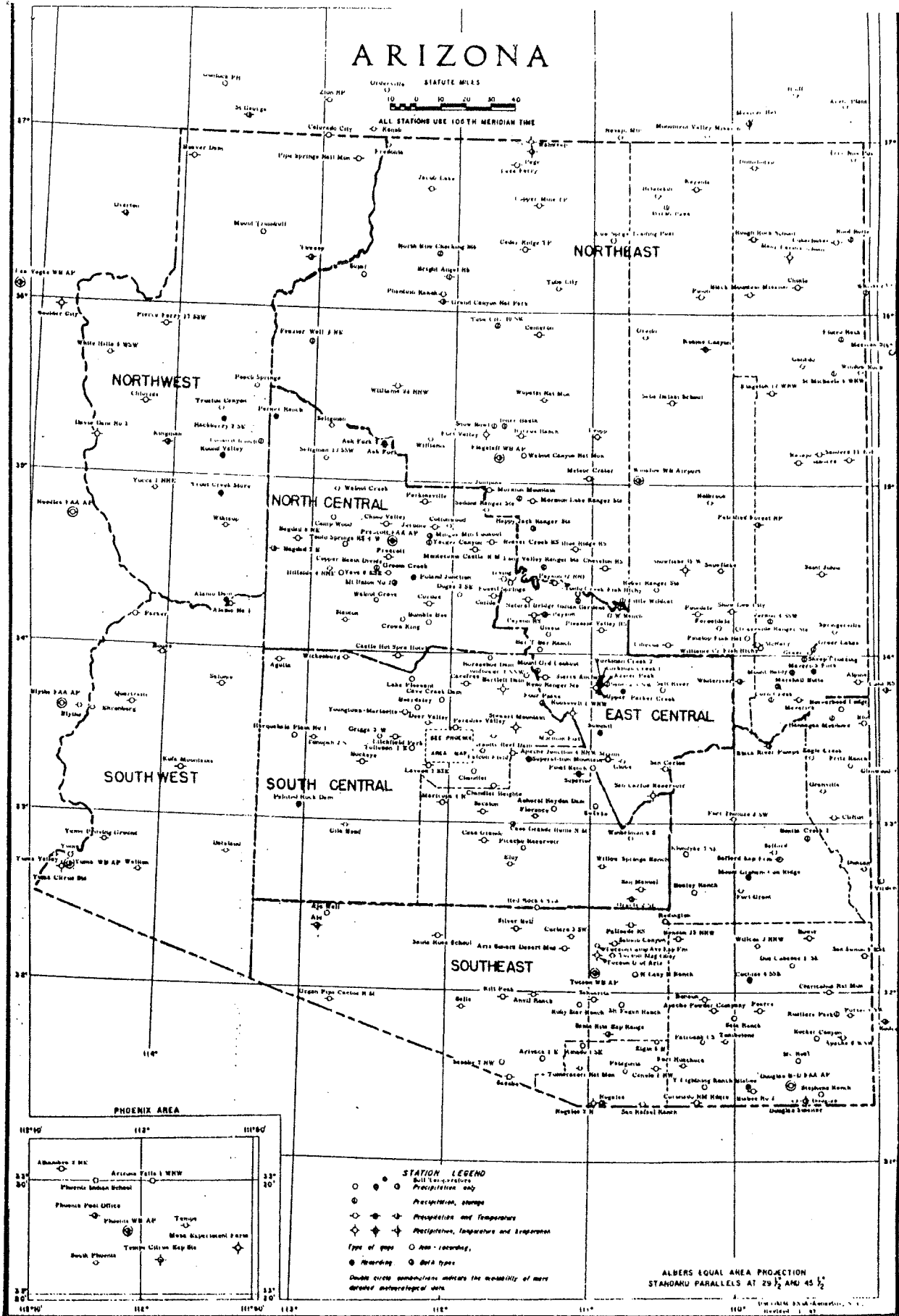
This report was prepared by a special Snow Load Committee of the Structural Engineers Association of Arizona, in cooperation with the Civil Engineering Department of Arizona State University. Much of the record search was done by John Nerison. Compilation and correlation of the data and preparation of the report was done under the direction of Mac Elliott.

After review by the board of directors of the Central Chapter of the S.E.A.A., and by other interested engineers, the report was circulated to building officials of the affected communities as well as to other pertinent government agencies, with requests for comments. Information received back generally correlated well with the report data. Where applicable, table values were adjusted slightly to reflect these comments, prior to publication.



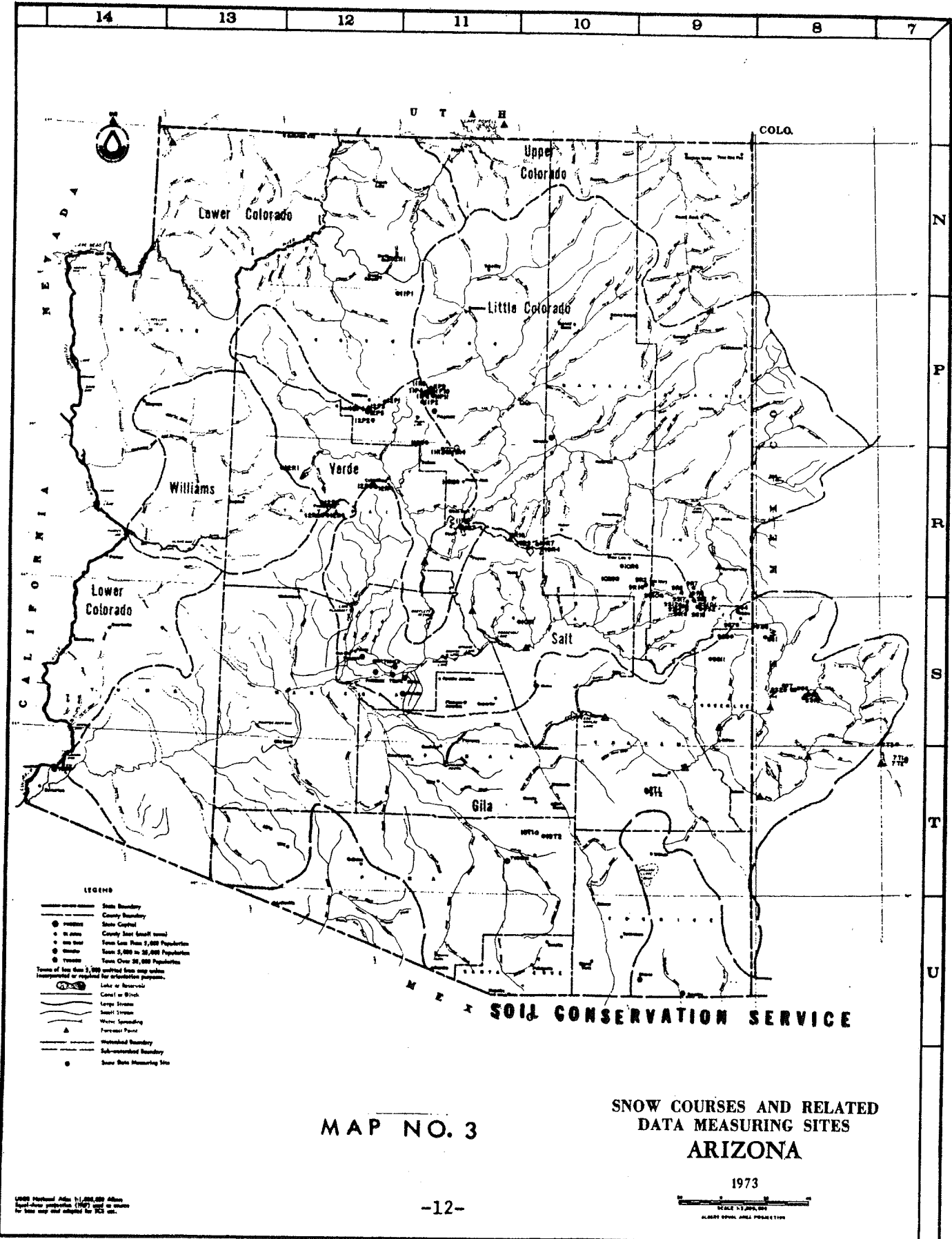
SNOW LOAD ZONES IN ARIZONA

MAP NO. 1



# WEATHER BUREAU REPORTING STATIONS

## MAP. NO 2



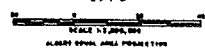
LEGEND

- State Boundary
- County Boundary
- State Capital
- County Seat (Small town)
- City Seat
- Town Less Than 5,000 Population
- Village 5,000 to 25,000 Population
- Precinct Town Over 25,000 Population
- Towns of less than 5,000 population. Sites may seldom be established or required for information purposes.
- Lake or Reservoir
- Canal or Ditch
- Large Stream
- Small Stream
- Water Spreading
- ▲ Forecast Point
- Watershed Boundary
- Sub-watershed Boundary
- Snow Data Measuring Site

MAP NO. 3

SNOW COURSES AND RELATED DATA MEASURING SITES ARIZONA

1973



1:250,000 National Atlas 1:1,000,000 Edition  
 Snow Data Measuring Sites 1967 and in use  
 for State and adopted for SCS use.



TABLE 1 SNOW ZONE I

Place	Elevation above Sea Level.	Month and Year of Maximum Snow.	Maximum Depth (inches)	Maximum Measured, Calculated, or Estimated Wt. of Snow on Ground (psf)	30 Year Basic Ground Snow Load (psf)	30 Year Basic Roof Snow Load (psf) **	Data Source †	Remarks
Betatakin	7286	12/67	38	10	20	18	HME	
Black Mountain Mission	6350	12/61	12	10*	20	18	WB	
Cameron	4165	12/67	18	8	12	12	HME	
Chinle	5538	12/67	11	9	16	16	HME	
Copper Mine T. P. (20 miles south of Page)	6380	12/60	23	16*	20	18	WB	
Dinnehotso	5020	12/67	--	8	16	16	HME	
Fort Defiance	6750	3/48	27.5	20*	20	18	WB	
Ganado	6350	12/67	24	10	20	18	HME	
Holbrook	5069	12/67	19	10	16	16	HME	
Jadito	6700	3/48	28	20*	20	18	WB	
Kaibato	6000	12/60	14	9*	20	18	WB	
Kayenta	5665	2/48	21	12*	16	16	WB	
Keams Canyon	6215	12/67	11	18	20	18	HME	
Leupp	4700	12/67	19	9	16	16	HME	
Lukachukai	6520	12/61	20	12*	20	18	WB	
Navajo (40 miles N.E. of Holbrook)	5580	12/67	--	8	16	16	HME	
Page	4270	12/67	9	7	12	12	HME	
Petrified Forest National Park	5460	11/31	20	12*	16	16	WB	
Pinon	6000	12/67	21	14	20	18	HME	
Saint Johns	5730	1/37	19	12*	16	16	WB	
Sanders 11 ESE	6250	12/67	20	8	20	18	HME	
Seba Dalkai School	5900	12/67	55	30*	25	25	WB	
Snowflake	5642	12/67	30	21	20	20	HME	
Tuba City	4936	12/67	20	7	16	16	HME	
Window Rock	6750	12/67	18	8	20	18	HME	
Winslow	4895	12/67	29	19	20	20	HME	
Wupatki National Monument	4908	12/67	32	13	16	16	HME	

† SCS = Soil Conservation Service. WB = Weather Bureau. HME = Elliott Report.

\* Estimated weight (as opposed to measured or calculated weight).

\*\* When roof is fully exposed to wind, 30 year Basic Roof Load may be further reduced 10% at elevations above 6000 ft., and 20% above 7500 ft. See Table 2 for example.

TABLE 2 SNOW ZONE II

	Elevation above Sea Level.	Month and Year of Maximum Snow	Maximum Depth (inches)	Maximum Measured, Calculated, or Estimated Wt. of Snow on Ground, (psf)	30 Year Basic Ground Snow Load (psf)	30 Year Basic Roof Snow Load (psf) **	Data Source †	Remarks	
Agassiz (10 miles N. of Flagstaff)	11,200	4/73	126	220*	--	--	SCS		
Baker Butte (15 mi. North of Payson)	7,300	3/73	77	124	110	100	SCS		
Baker Butte #2 (15 mi. North of Payson)	7,700	3/73	97	160	145	115	SCS		
Bill Williams Intermediate (6 mi. So. Williams)	8,550	4/73	76	145	130	105	SCS		
Bill Williams Summit (6 mi. So. Williams)	8,950	3/73	108	164	150	120	SCS		
Bright Angel R.S. (No. Rim of Grand Canyon)	8,400	3/52	77	128	120	95	SCS		
Burrus Ranch (15 mi. N.E. of Flagstaff)	6,800	12/67	40	22	25	22	HME		
Canyon Creek #2 (13 mi. S.W. of Heber)	7,500	3/73	48	78	70	55	SCS		
Canyon Point (17 mi. S.W. of Heber)	7,600	3/73	59	90	80	65	SCS		
Chalender (7 mi. E. Williams)	7,100	3/73	44	62	55	50	SCS		
Chevelon R.S. (30 mi. N.E. of Payson)	7,006	12/67	52	39	50	45	HME		
Cibecue	4,950	12/67	16	31	30	30	HME		
Doyle Saddle (7 mi. No. of Flagstaff)	10,900	4/73	--	200*	--	--	SCS		
Flagstaff Airport	6,993	12/67	83	37	40	35	HME	May be light for some areas of city. e.g. see Fort Valley	
Fort Valley (7 mi. No. of Flagstaff)	7,350	2/49	42	60	55	50	SCS		
Grand Canyon (10 mi. S.E. of Village)	7,500	3/73	33	55	50	40	SCS		
Grand Canyon National Park	6,950	1/49	38	48*	45	40	WB		
Happy Jack (35 mi. So. of Flagstaff)	7,630	3/73	72	105	95	75	SCS		
Heber (12 mi. S.W. of Heber)	7,600	3/73	53	84	75	60	SCS		
Heber Ranger Station	6,590	12/67	48	44	40	36	HME		
Inner Basin #1	10 miles North of Flagstaff	10,000	4/73	125	228	205	165	SCS	
Inner Basin #2		9,750	4/73	95	162	145	115	SCS	
Inner Basin #3		10,250	4/73	--	240*	--	--	SCS	
Jacob Lake	7,920	4/73	60	85*	80	64	WB		
Mormon Lake	20 mi. So. of Flagstaff	7,350	2/49	73	116	105	95	SCS	
Mormon Mountain	7,500	3/73	78	125	110	90	SCS		
Natural Bridge (10 mi. N.W. of Payson)	4,607	12/67	--	36	30	30	HME		
Newman Park (15 mi. S.W. of Flagstaff)	6,750	3/73	57	75	65	60	SCS		
Payson	4,913	12/67	48	47	40	40	HME		
Payson (12 mi. NNE)	5,500	12/67	42	55	50	50	HME		
Pleasant Valley R.S. (20 mi. E. of Payson)	5,050	12/67	27	26	25	25	HME		
Snow Bowl #1	10 mi. No. of Flagstaff	10,260	4/73	85	163*	--	--	SCS	
Snow Bowl #2	11,000	4/73	130	222*	--	--	SCS		
Tonto Creek Fish Hatchery (15 mi. N.E. of Payson)	6,280	12/67	58	52	45	40	HME		
Walnut Canyon (10 mi. E. of Flagstaff)	6,685	12/67	54	43	40	36	HME		
White Horse Lake Junction (10 mi. So. of Williams)	7,180	3/73	57	86	75	70	SCS		
Williams	6,750	1/30	53	42*	40	36	WB		
Williams Ski Run (5 mi. So. of Williams)	7,720	4/73	70	128	115	105	SCS		
Young	5,200	2/44	25	20*	20	20	WB		

† SCS = Soil Conservation Service. WB = Weather Bureau. HME = Elliott Report.

\* Estimated weights, (as opposed to measured or calculated weights).

\*\* When roof is fully exposed to wind, 30 year Basic Roof Load may be further reduced 10% at elevations above 6,000 ft., and 20% above 7,500 ft. e.g. Could reduce Flagstaff A.P. to 35 x .9 = 32 psf. Could reduce Bright Angel to 95 x .8 = 76 psf.

TABLE 3 SNOW ZONE III

Place	Elevation above Sea Level.	Month and Year of Maximum Snow	Maximum Depth (inches)	Maximum Measured, Calculated, or Estimated Wt. of Snow on Ground. (pcf)	30 Year Basic Ground Snow Load (pcf)	30 Year Basic Roof Snow Load (pcf) **	Data Source †	Remarks
Alpine	8,020	12/67	60	36	50	40	HME	
Baldy (Sheep Crossing)	9,125	3/62	47	90	90	70	SCS	
Baldy #2 } 20 mi. SE	9,750	4/73	83	173	155	125	SCS	
Baldy #3 } of McNary	10,950	4/73	117	245	--	--	SCS	
Beaverhead Lodge (10 mi. S. of Alpine)	8,000	1/68	38	65	60	50	SCS	
Blue	5,760	12/67	42	36	35	35	HME	
Cheese Springs (18 mi. E. of McNary)	8,600	3/73	44	64	60	50	SCS	
Coronado Trail (4 mi. SW of Alpine)	8,000	2/49	38	64	60	50	SCS	
Forestdale (5 mi. SW of Show Low)	6,430	1/68	25	48	40	36	SCS	
Ft. Apache (17 mi. E. of McNary)	9,160	3/62	58	90	90	70	SCS	This is not the town of Ft. Apache.
Frisco Divide (15 mi. SE of Alpine)	8,000	1/68	31	50	50	40	SCS	
Greer	8,490	12/67	54	36	50	40	HME	
Hannagan Meadows	9,090	3/73	67	113	100	80	SCS	
Hawley Lake	8,300	12/67	91	57	95	75	HME	2 Sets Data Illustrate Different Wt./Depth Ratios
Hawley Lake	8,300	4/73	45	103	95	75	SCS	
Lakeside R.S.	6,700	12/67	52	36	40	36	HME	
Maverick Fork (20 mi. SE of McNary)	9,150	3/73	64	106	95	75	SCS	
McNary (2 mi. W. of McNary)	7,200	3/73	41	60	55	50	SCS	
McNary	7,320	1/37	71	60*	55	50	WB	
Milk Ranch (5 mi. SW of McNary)	7,000	3/73	32	42	45	40	SCS	
Mt. Ord (15 SE of McNary)	11,200	4/73	134	273*	--	--	SCS	
Nutrioso (3 mi. N. of Alpine)	8,500	2/49	34	47	50	40	SCS	
Pinedale (15 mi. W. of Showlow)	6,500	1/37	42	42*	40	36	WB	
Pinetop Fish Hatchery	7,200	12/67	54	52	50	45	HME	
Show Low	6,412	12/67	41	31	35	32	HME	
Smith Cienega (15 mi. SE of McNary)	10,050	3/73	97	191*	--	--	SCS	
Springerville	7,060	2/48	28	35*	35	32	WB	
State Line (7 mi. SE of Alpine)	8,000	1/68	33	42	50	40	SCS	
Sunrise Summit (17 mi. SE of McNary)	10,600	4/73	80	147	--	--	SCS	
Whiteriver	5,280	1/60	21	21*	25	25	WB	
Williams Creek Fish Hatchery (2 mi. SE of McNary)	6,960	1/49	52	55*	50	45	WB	
Wilson Lake (13 mi. E. of McNary)	9,000	3/73	73	112	100	80	SCS	

† SCS = Soil Conservation Service. WB = Weather Bureau. HME = Elliott Report.

\* Estimated weights, (as opposed to measured or calculated weights).

\*\* When roof is fully exposed to wind, 30 year Basic Roof Load may be further reduced 10% at elevations above 6,000 ft., and 20% above 7,500 ft. See Table 2 for example.

TABLE 4

SNOW ZONE IV

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Place	Elevation above Sea Level.	Month and Year of Maximum Snow.	Maximum Depth (inches)	Maximum Measured, Calculated, or Estimated Wt. of Snow on Ground. (pcf)	30 Year Basic Ground Snow Load (pcf)	30 Year Basic Roof Snow Load (pcf) **	Data Source †	Remarks
Ash Fork	5,200	12/67	38	14	20	20	HME	
Bagdad (8 mi. NE)	4,240	12/67	15	18	20	20	HME	
Beaver Creek R.S. (12 mi. S. of Sedona)	3,830	12/67	26	13	20	20	HME	
Call of the Canyon (10 mi. N. of Sedona)	5,329	2/44	60	48*	50	50	WB	
Camp Wood (30 mi. NW of Prescott)	5,700	2/49	33	45	40	40	SCS	
Chino Valley	4,750	12/67	22	16	20	20	HME	
Clifton	3,465	12/67	20	18	20	20	HME	
Copper Basin Divide (7 mi. SW of Prescott)	6,720	12/67	37	59	55	50	SCS	
Cordes Junction	3,773	12/67	15	23	20	20	HME	
Cottonwood	3,360	12/67	26	26	20	20	HME	
Crown King	6,000	12/67	54	60	55	50	HME	
Eagle Creek (20 mi. SW of Hannagan Meadows)	5,100	12/67	35	12	20	20	HME	
Fraziers Well 4 mi. NE (25 mi. NE Peach Springs)	6,500	2/44	22	30*	40	36	WB	
Fredonia	4,675	1/44	20	20*	20	20	WB	
Gaddes Canyon (20 mi. NE of Prescott)	7,600	3/73	72	108	100	90	SCS	
Globe	3,540	1/37	24	20*	20	20	WB	
Groom Creek	6,100	2/44	73	60*	50	45	WB	
Hilltop (55 mi. NE Peach Springs)	5,700	2/44	26	26	35	35	WB	
Highland Pines (7 mi. W. of Prescott)	7,000	12/67	48	52	55	50	HME	
Iron Springs (7 mi. W. of Prescott)	6,200	2/49	34	57	50	45	SCS	
Jerome	5,245	12/67	40	31*	30	30	HME	
Junipine (8 mi. N. of Sedona)	5,124	3/45	70	60*	50	50	WB	
Kingman	3,539	12/32	14	15*	20	20	WB	
Miami	3,560	12/67	16	21*	20	20	HME	
Mingus Mountain (20 mi. NE Prescott)	7,100	2/49	30	56	55	50	SCS	
Montezuma's Castle National Monument	3,180	12/67	19	16	20	20	HME	
Peach Springs	4,970	12/67	27	13	20	20	HME	
Pipe Springs National Monument	4,920	1/73	18	20*	20	20	WB	
Prescott	5,410	1/30	46	30	30	30	WB	
Sedona R.S.	4,223	12/67	15	18	20	20	HME	
Seligman	5,230	2/32	28	15*	20	20	HME	
Sierra Ancha (25 mi. N. of Miami)	5,100	12/67	30	47	40	40	HME	
Stanton (15 mi. N. of Wickenburg)	3,480	12/67	10	13	20	20	HME	
Tuwcep (50 mi. SW of Jacob Lake)	4,775	12/41	17	17*	20	20	WB	
Walnut Creek R.S. (25 mi. SW of Ash Fork)	5,090	3/45	19	25*	20	20	WB	
Walnut Grove (16 mi. S. of Prescott)	3,764	2/44	25	25*	20	20	WB	
Workman Creek (30 mi. N. of Globe)	6,900	3/73	60	105	90	81	SCS	
Yaeger Canyon (10 mi. SW of Cottonwood)	6,000	1/45	30	30*	35	32	WB	
Yarnell	4,848	1/37	28	25*	25	25	WB	

† SCS = Soil Conservation Service. WB = Weather Bureau. HME = Elliott Report.

\* Estimated weights, (as opposed to measured or calculated weights).

\*\* When roof is fully exposed to wind, 30 year Basic Roof Load may be further reduced 10% at elevations above 6,000 ft., and 20% above 7,500 ft. See Table 2 for example.

TABLE 5 SNOW ZONE V

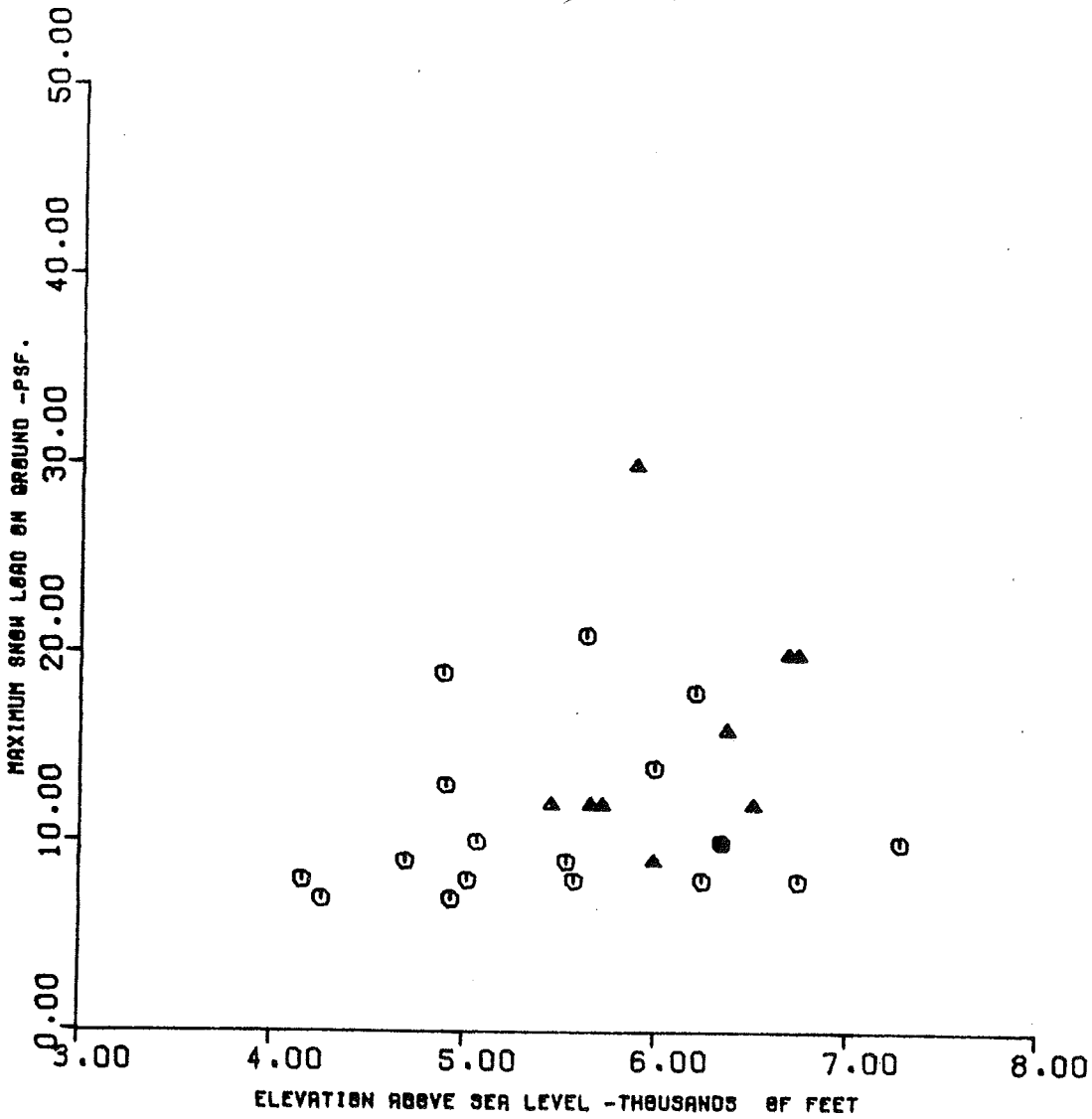
Place	Elevation above Sea Level.	Month and Year of Maximum Snow.	Maximum Depth (inches)	Maximum Measured, Calculated or Estimated Wt. of Snow on Ground. (psf)	30 Year Basic Ground Snow Load (psf)	30 Year Basic Roof Snow Load (psf) **	Data Source †	Remarks
Bear Wallow (20 mi. NE of Tucson)	8,100	2/68	40	88	88	79	SCS	
Bisbee	5,440	12/67	24	17	20	20	HME	
Bisbee #2 (3 mi. SE of Bisbee)	5,020	1/49	20	10*	20	20	WB	
Chiricahua National Monument	5,300	12/67	28	16	20	20	HME	
Crazy Horse (14 mi. SW of Safford)	10,200	3/66	108	198*	--	--	SCS	
Dos Cabezas (15 mi. SE of Willcox)	5,100	12/67	12	16	20	20	HME	
Douglas	4,040	12/67	--	5	12	12	HME	
Ft. Grant	4,875	12/67	10	13	20	20	HME	
Ft. Huachuca	4,664	12/67	7	13	20	20	HME	
High Peak (14 mi. SW of Safford)	10,500	3/66	120	218*	--	--	SCS	
Kitt Peak	6,875	12/67	35	44	44	44	HME	
Nogales	3,800	12/71	--	10	12	12	HME	
Oracle 2 mi. SE	4,540	1/37	26	21*	20	20	WB	
Palisade R.S. - Mount Lemmon	7,945	2/66	86	86*	80	72	WB	
Patagonia	4,044	12/67	--	8*	12	12	HME	
Pearce (20 mi. NE of Tombstone)	4,420	12/67	4	5	12	12	HME	
Final Ranch (5 mi. E. of Superior)	4,520	12/67	25	26*	20	20	HME	
Portal 4 mi. SW	5,390	12/67	31	26	20	20	HME	
Rose Canyon (20 mi. NE of Tucson)	7,300	2/66	53	71	70	63	SCS	
Sala Ranch (10 mi. NE of Tombstone)	5,190	12/67	11	10	20	20	HME	
Safford	2,900	12/67	14	10	12	12	HME	
San Manuel	3,560	12/67	5	10	12	12	HME	
Santa Rita Experimental Range (25 mi. S. of Tucson)	4,300	12/67	--	10	12	12	HME	
Tombstone	4,540	12/67	--	10*	20	20	HME	
Willcox 3 mi. NNW	4,190	12/67	6	8	12	12	HME	

† SCS = Soil Conservation Service. WB = Weather Bureau. HME = Elliott Report.

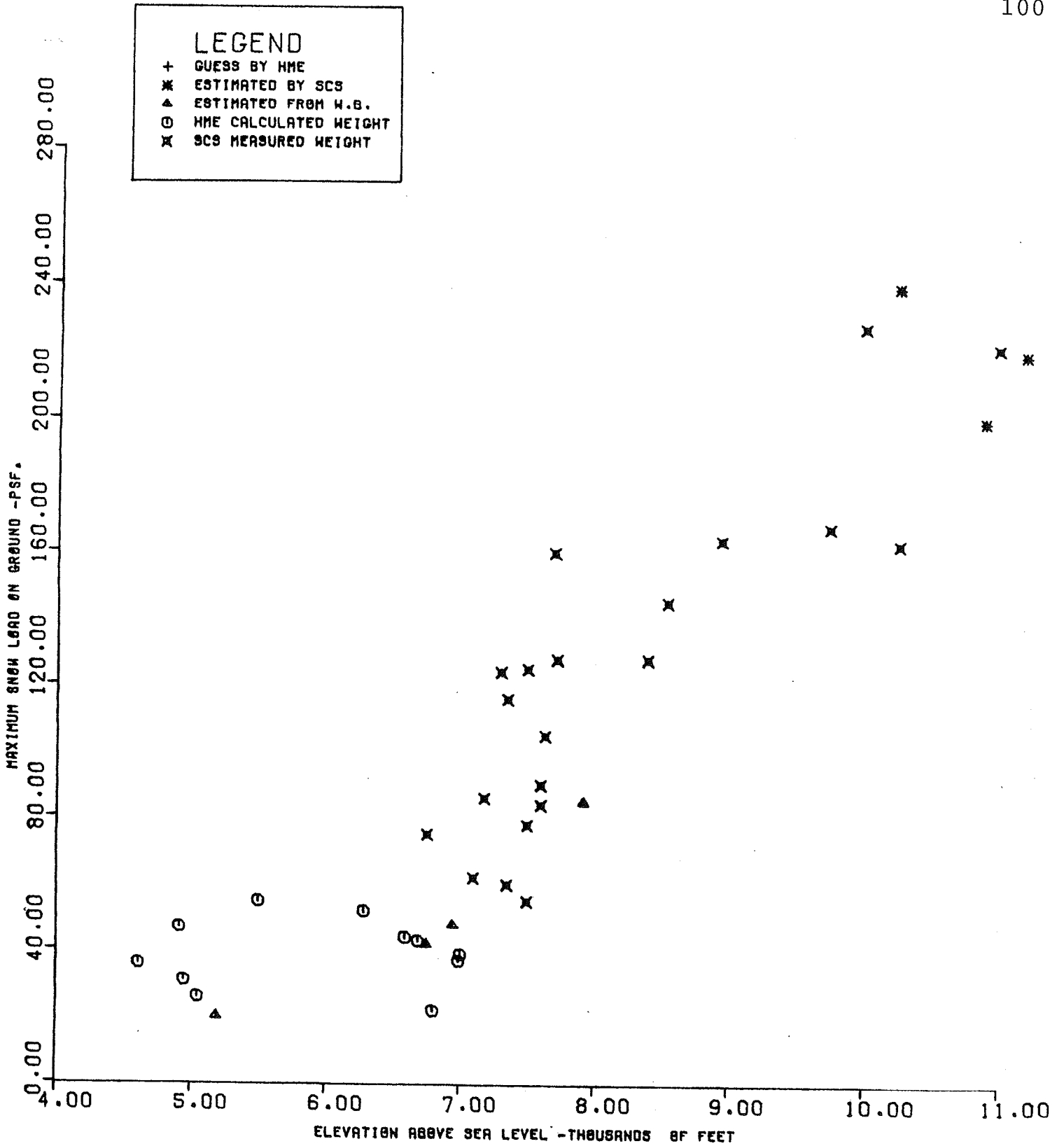
\* Estimated weights, (as opposed to measured or calculated weights).

\*\* When roof is fully exposed to wind, 30 year Basic Roof Load may be further reduced 10% at elevations above 7,000 ft. See Table 2 for example.

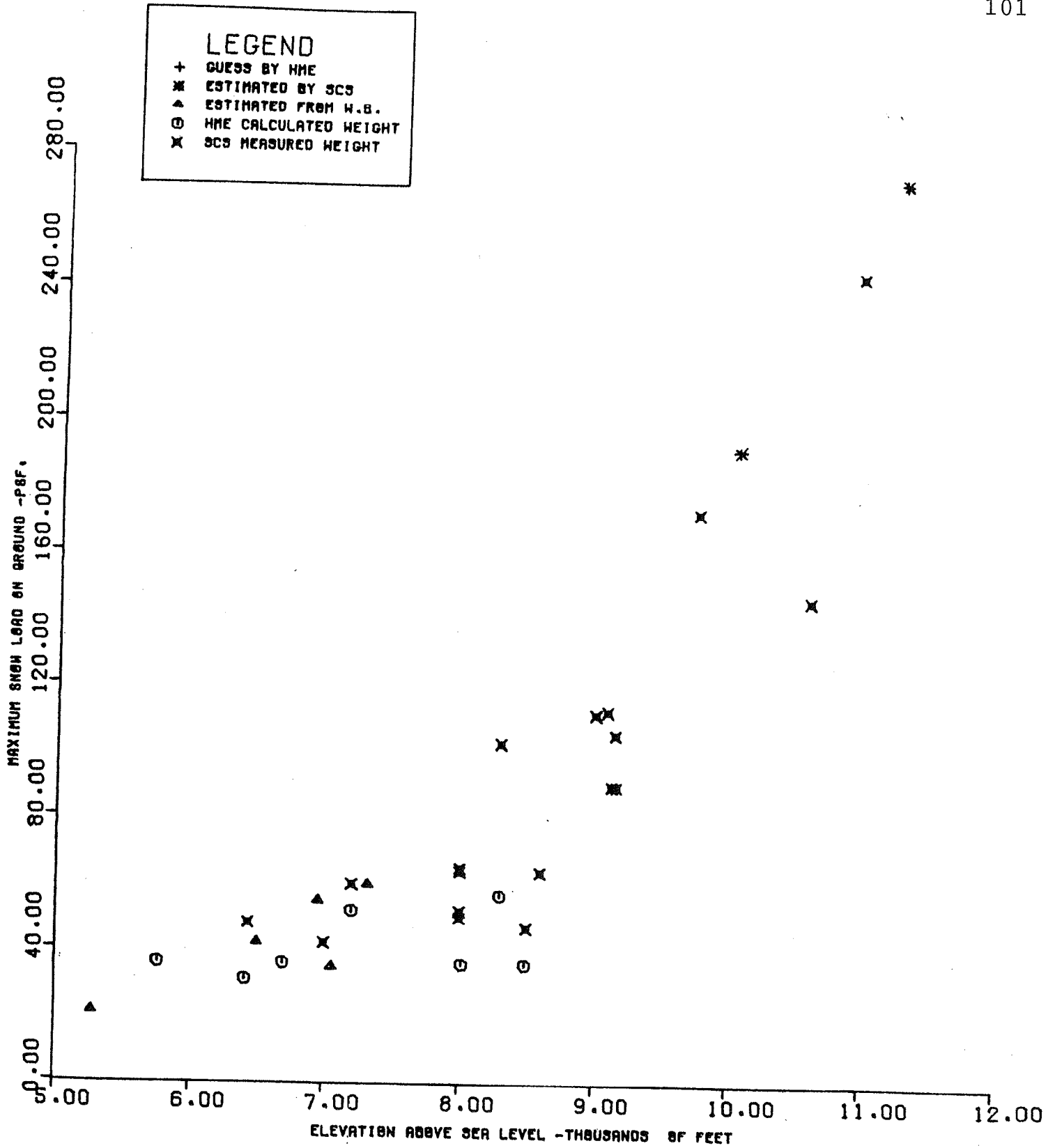
**LEGEND**  
+ GUESS BY HME  
\* ESTIMATED BY SCS  
▲ ESTIMATED FROM W.B.  
⊙ HME CALCULATED WEIGHT  
× SCS MEASURED WEIGHT



MAXIMUM GROUND SNOW LOADS VS ELEVATIONS-ZONE I  
FIGURE 1

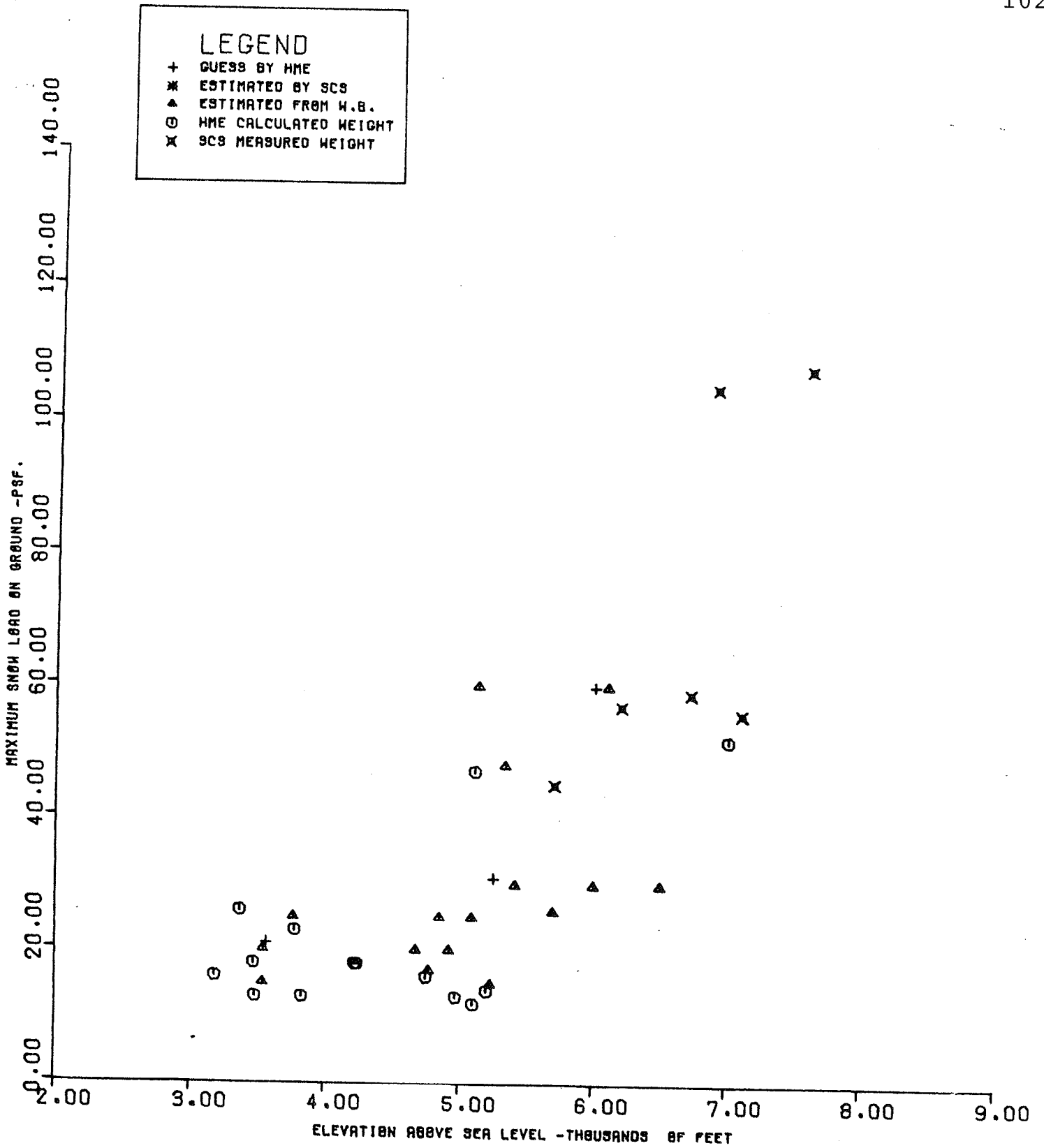


MAXIMUM GROUND SNOW LOADS VS ELEVATIONS-ZONE II  
 FIGURE 2

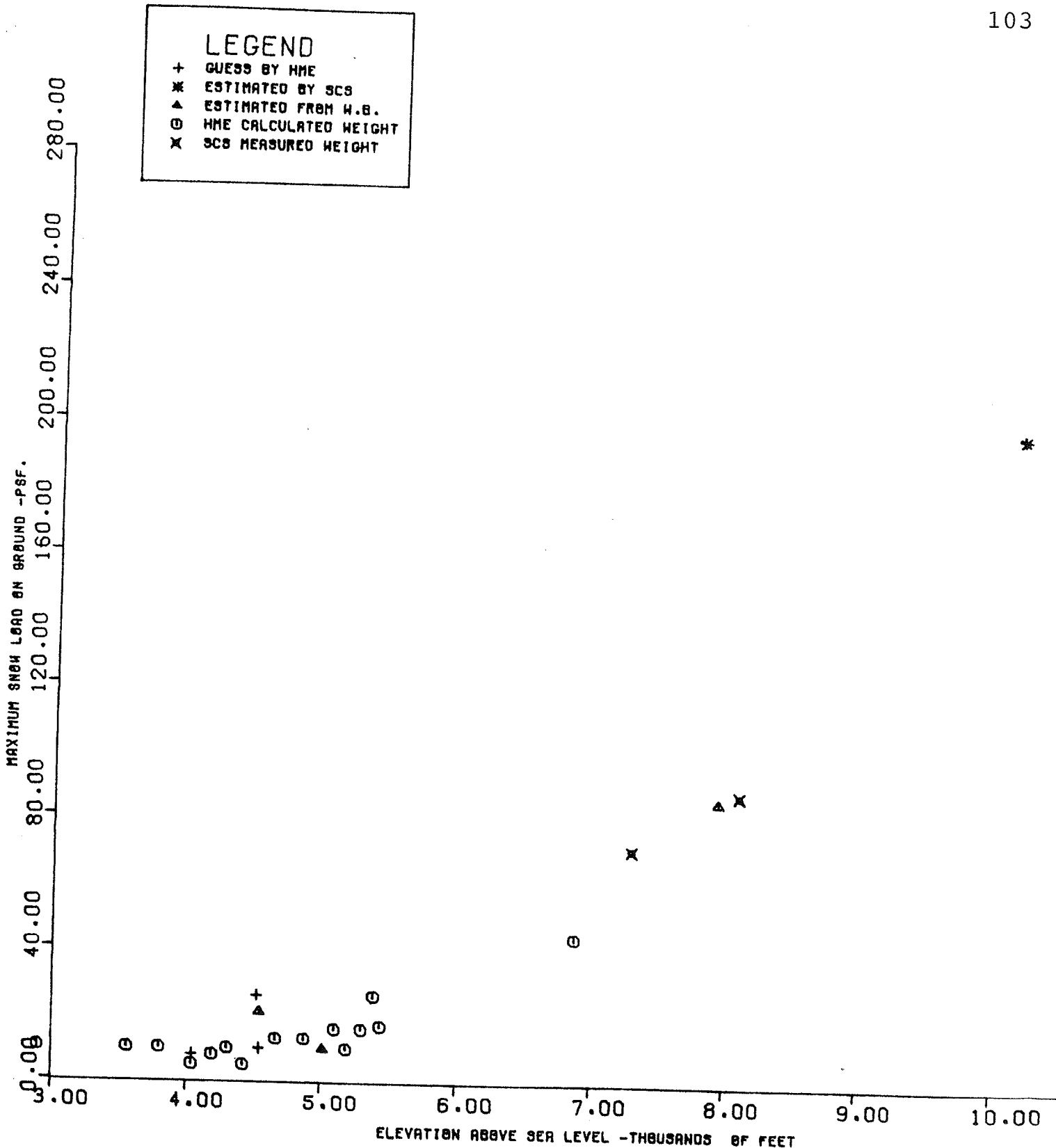


MAXIMUM GROUND SNOW LOADS VS ELEVATIONS-ZONE III  
 FIGURE 3





MAXIMUM GROUND SNOW LOADS VS ELEVATIONS-ZONE IV  
 FIGURE 4



MAXIMUM GROUND SNOW LOADS VS ELEVATIONS-ZONE V  
FIGURE 5

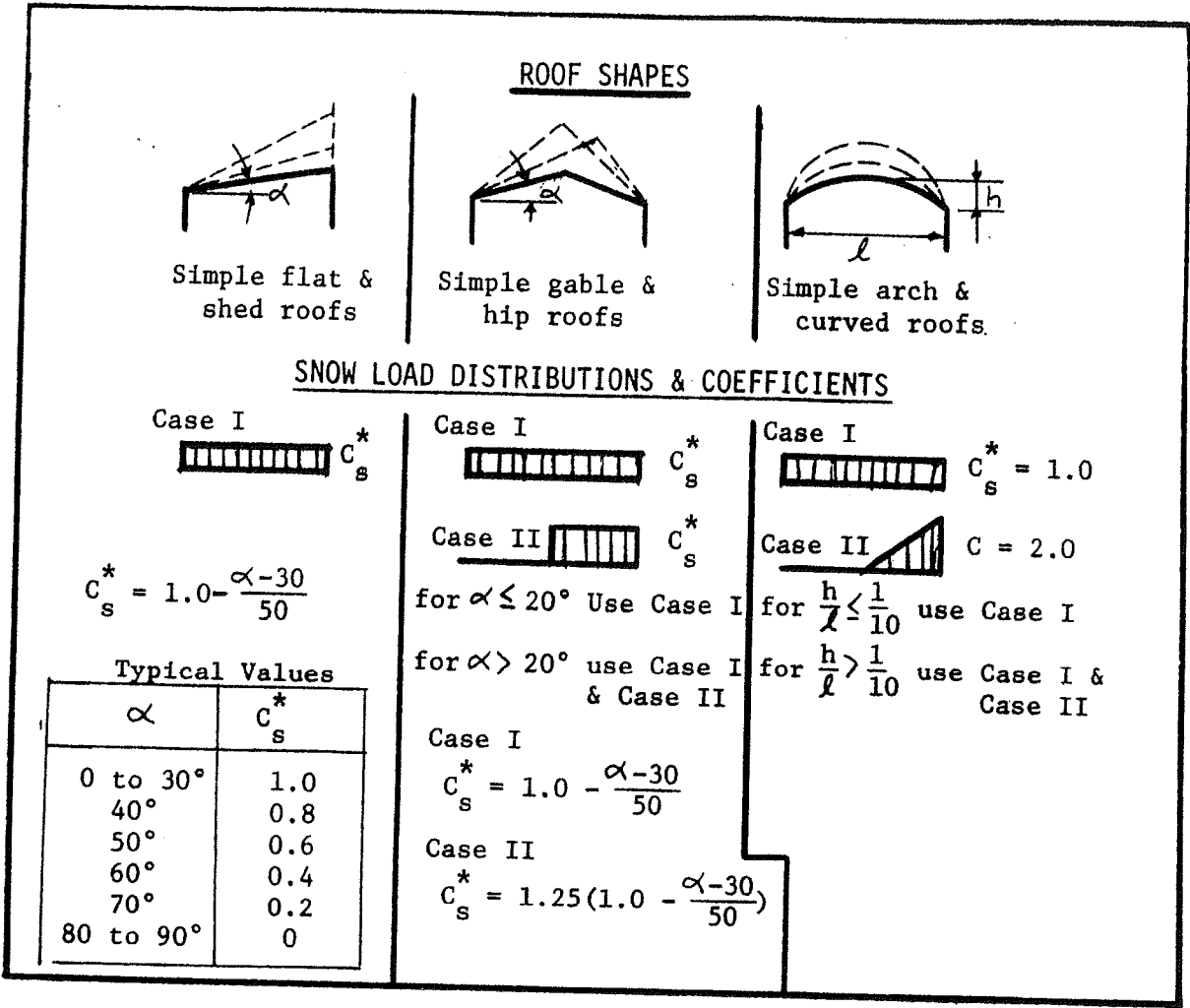


Fig C2-1  
Flat & Shed Roofs

Fig C2-2  
Gable or Hip Roofs

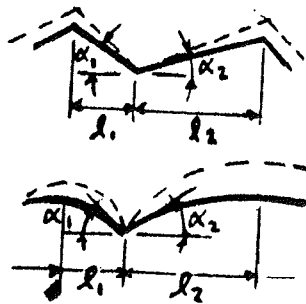
Fig C2-3  
Arch Roofs

**Notes:**

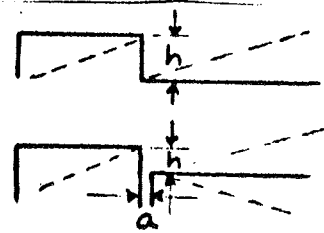
1. In Figs C2-1 & C2-2 the term  $\frac{\alpha - 30}{50}$  is only valid for slopes greater than 30°.
2.  $C_s^*$  = coefficient to be applied to Basic Roof Snow Load.
3.  $C$  = coefficient to be applied to Basic Ground Snow Load.

**MODIFICATIONS DUE TO ROOF SLOPES**  
**AND UNBALANCED LOADS**

ROOF SHAPES



Valley Areas of 2-span & multi span sloped or curved roofs



Lower level of multi-level roofs (where upper roof is part of the same building or on an adjacent building not more than 15 ft. away.)

SNOW LOAD DISTRIBUTIONS & COEFFICIENTS

Case I  $C_s^*$  (uniform load)

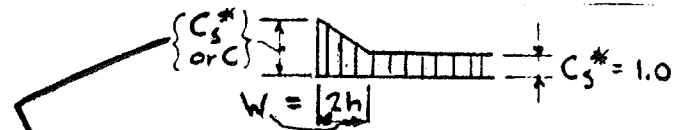
$$C_s^* = 1.0 - \frac{\alpha - 30}{50}$$

valley  $C = 1.0$

Case II  $C = 0.5$

Case III  $C = 1.5$   
valley  $C = 0.5$   
 $\beta = \frac{\alpha_1 + \alpha_2}{2}$

for  $\beta \leq 10^\circ$  use Case I  
for  $10^\circ < \beta < 20^\circ$  Use Case I & II  
for  $\beta \geq 20^\circ$  use Case I, II & III



$C = 15 \frac{h}{g}$  except:

when  $15 \frac{h}{g} \leq 1.0$ , use  $C_s^* = 1.0$

when  $15 \frac{h}{g} > 3.0$ , use  $C = 3.0$

$W = 2h$  except:

when  $h < 5\text{ft}$  use  $W = 10$   
when  $h > 15\text{ft}$  use  $W = 30$

$h$  = difference of roof heights in ft.  
 $g$  = Basic Ground Snow Load in psf  
 $w$  = width of drift from higher bldg. in ft.  
 $a$  = distance between bldgs. < 15ft.

For loads on upper roof use Figs. C2-1 to C2-4.

Fig. C2-4

Valley Areas of 2-span & Multi Span Sloped or Curved Roofs.

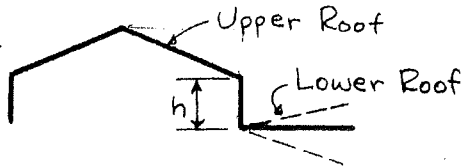
Fig C2-5

Lower Roof of Multi-Level Roofs.

Notes:

1. In Fig C2-4 the term  $\frac{\alpha - 30}{50}$  is only valid for slopes greater than  $30^\circ$ .
2.  $C_s^*$  = coefficient to be applied to Basic Roof Snow Load.
3.  $C$  = coefficient to be applied to Basic Ground Snow Load.

MODIFICATIONS DUE TO ROOF VALLEYS AND MULTI LEVEL ROOFS.



Lower of multi-level roofs with upper roof sloped towards lower roof.



Roof areas adjacent to projections & obstructions on roofs.

SNOW LOAD DISTRIBUTIONS & COEFFICIENTS

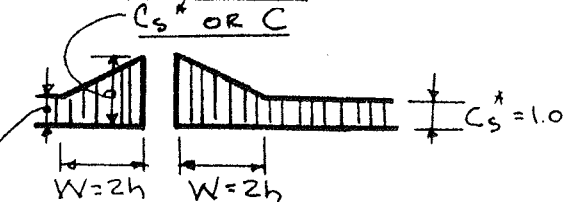
$W_1$  = Total load from sliding snow

Drift load per Fig C2-5

Design lower roof for loads according to Fig. C2-5, plus  $W_1$ .

(Designer must use judgement in estimating  $W_1$ , the maximum probable weight of snow melt or sliding snow from roof above. As a guide only, for average conditions,  $W_1$  could equal 50% of the maximum total design load on the portion of the upper roof which slopes towards the lower roof.)

Design upper roof for loads according to Figs. C2-1 to C2-4.



$C_s^* = 1.0$

$C = 10 \frac{h}{g}$  except:

when  $10 \frac{h}{g} < 1.0$ , use  $C_s^* = 1.0$

when  $10 \frac{h}{g} > 2.0$ , use  $C = 2.0$

when  $l < \frac{g}{6}$  use  $C_s^* = 1.0$

$W = 2h$  except:

when  $h < 5\text{ft}$  use  $W = 10$

when  $h > 15\text{ft}$  use  $W = 30$

$h$  = height of projection in ft.

$g$  = Basic Ground Snow Load in psf

$w$  = width of snow drift in ft.

$l$  = length of projection in ft.

Fig C2-6

Lower Of Multi-Level Roofs With Upper Roof Sloped Towards Lower Roof.

Fig C2-7

Areas Adjacent To Roof Projections.

Notes

1.  $C_s^*$  = coefficient to be applied to Basic Roof Snow Load.
2.  $C$  = coefficient to be applied to Basic Ground Snow Load.

MODIFICATIONS DUE TO MULTI LEVEL ROOFS & ROOF PROJECTIONS.

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